DESIGN AND CONSTRUCTION SHALL BE IN ACCORDANCE WITH THE PROVISIONS OF THE 2017 FLORIDA BUILDING CODE. BISK CATEGORY		 FOUNDATIONS AND EARTHWORK REMOVE EXISTING SURFICIAL TOP SOIL AND VEGETATION FROM WITHIN THE BUILDING AREA AND A MINIMUM OF TEN FEET BEYOND EXCAVATE MATERIAL TO PROPOSED SUB-ONLORADE SUBCRADE OPDODED UNITH A HEAVY DUPPED 	WOOD TRUSSES SHALL BE DESIGNED IN ACCORDANCE WITH THE CURRENT EDITIONS OF "DESIGN SPECIFICATIONS FOR METAL PLATE CONNECTED WOOD TRUSSES" BY TRUSS PLATE INSTITUTE (TPI) AND	
 DESIGN LOADS AND DATA SUPERIMPOSED LOADS 	II	TIRED VEHICLE. SOILS WHICH HEAVE, PUMP, OR DO NOT READILY COMPACT SHALL BE EXCAVATED AND REPLACED WITH ENGINEERED FILL.	"NATIONAL DESIGN SPECIFICATIONS FOR STRESS GRADE LUMBER AND ITS FASTENINGS" BY NATIONAL FOREST PRODUCTS ASSOCIATION.	
I YPICAL FLOOR DEAD LIVE	(REFER TO PLAN) (REFER TO PLAN)	SUBGRADE PREPARATION FOR FOOTINGS SHALL CONSIST OF EXCAVATION TO REQUIRED ALLOWABLE BEARING CAPACITY SOILS AT OR NEAR DESIGN FOOTING ELEVATIONS. WHERE UNSUITABLE SOIL IS ENCOUNTERED AT NOMINAL	ROOF TRUSSES SHALL BE DESIGNED FOR THE FOLLOWING LOADS: TOP CHORD LIVE LOAD 20 PSF TOP CHORD DEAD LOAD 15 PSE	ZONE WIND AREA (SF) 0° TO 7° 7° TO 27° 27° TO 45° ZONE WIND AREA (SF)
ROOF LOADS DEAD	25 PSF	BEARINGDEPTH, SEE OVER EXCAVATION DETAIL.	BOTTOM CHORD LIVE LOAD 10 PSF (NOT CONCURRENT WITH TOP CHORD) BOTTOM CHORD DEAD LOAD 10 PSF	$\begin{array}{c ccccccccccccccccccccccccccccccccccc$
ROOF LIVE MECHANICAL EQUIPMENT, PIPING AND ROOF TOP AHU'S	20 PSF AS NOTED ON DRAWINGS	ALL COMPACTION REQUIREMENTS REFER TO % OF MAXIMUM DRY DENSITY PER ASTM D-1557 MODIFIED PROCTOR. GRANULAR STRUCTURAL FILL BENEATH FOOTINGS SHALL BE PLACED IN LAYERS NO MORE THAN 8" THICK, AND EACH	FLOOR TRUSSES SHALL BE DESIGNED FOR THE FOLLOWING LOADS:	$\begin{array}{c ccccccccccccccccccccccccccccccccccc$
 WIND DATA ULTIMATE DESIGN WIND SPEED (3 SECOND GUST) NOMINAL DESIGN WIND SPEED (3 SECOND GUST) 	162MPH 125 5 MPH	LAYER SHALL BE COMPACTED TO 95%. COHESIVE FILL APPROVED BY THE GEOTECHNICAL CONSULTANT SHALL BE PLACED IN LAYERS NO THICKER THAN 8", AND EACH LAYER SHALL BE COMPACTED TO 95%. MOISTURE CONDITION FILL MATERIALS AS REQUIRED TO OBTAIN PROPER COMPACTION. COHESIVE SOILS OF GRANULAR SOILS WITH A	TOP CHORD LIVE LOADREFER TO BUILDING DESIGN LIVE LOADSTOP CHORD DEAD LOAD25 PSF	1 50 11.9 32.3 15.7 29.4 29.4 30.3 4 50 28.2
BUILDING ENCLOSURE EXPOSURE	ENCLOSED C	SIGNIFICANT PERCENT OF COHESIVE FINES SHALL BE CONDITIONED TO WITHIN 3% OF OPTIMUM MOISTURE CONTENT AT COMPACTION.	BOTTOM CHORD LIVE LOAD 10 PSF (NOT CONCURRENT WITH TOP CHORD) BOTTOM CHORD DEAD LOAD 10 PSF	1 100 11.1 31.4 14.0 28.5 28.5 28.5 4 100 26.8 2 10 14.0 57.6 19.8 54.7 31.4 40.1 5 10 31.4
WIND IMPORTANCE FACTOR (Iw) WIND DIRECTIONALITY FACTOR (Kd)	1.0 0.85	FOR GENERAL INFORMATION AND SPECIFIC RECOMMENDATIONS AND REQUIREMENTS PERTAINING TO THE PROJECT	IN ADDITION TO THE LOADS STATED ABOVE THE TRUSSES SHALL BE DESIGNED FOR ANY MECHANICAL, AND/OR ANY SPECIAL LOAD CONDITIONS AS SHOWN ON STRUCTURAL PLANS AND AS REQUIRED BY THE FLORIDA BUILDING	2 20 13.1 51.5 18.0 50.3 30.5 38.4 5 20 30.0
TOPOGRAPHIC FACTOR (Kzt) GUST FACTOR (BUILDING IS RIGID) INTERNAL RRESSURE COEFFICIENT (CCDi)	1.0 0.85 + 0.18	SITE, REFER TO THE PROJECT GEOTECHNICAL REPORT PREPARED BY UNIVERSAL ENGINEERING SCIENCES DATED NOVEMBER 2, 2017. UES REPORT #0530.1700252.0000	CODE. GC AND TRUSS DESIGNER SHALL REVIEW DRAWINGS FROM ALL DISCIPLINES TO ENSURE ALL ISOLATED LOADS FROM UTILITIES AND MECHANICALS, EITHER HUNG OR FLOOR SUPPORTED, ARE CONSIDERED IN THE	2 50 11.9 43.4 15.7 44.5 29.4 36.1 5 50 28.2 2 100 11.1 27.2 14.0 40.1 28.5 34.2 5 100 26.8
ANALYSIS PROCEDURE COMPONENTS AND CLADDING	± 0.16 CHAPTER 28 SEE ADJACENT TABLE	COLUMNS, PIERS, AND SPREAD FOOTINGS ARE CENTERED ON GRIDLINES UNLESS NOTED OTHERWISE. CONTINUOUS FOOTINGS ARE CENTERED ON WALLS ABOVE UNLESS NOTED OTHERWISE.	DESIGN OF BOTH FLOOR AND ROOF TRUSSES.	2 100 11.1 37.2 14.0 40.1 28.3 54.3 5 100 26.6 3 10 14.0 86.7 19.8 80.9 31.4 40.1
MINIMUM NET UPLIFT INTERIOR SPACES	20 PSF (STRENGTH)	BACKFILL UNIFORMLY ON EACH SIDE OF FOUNDATION WALLS, GRADE BEAMS AND OTHER SIMILAR ELEMENTS. DO NOT	ROOF TRUSSES SHALL HAVE A MAXIMUM LIVE LOAD DEFLECTION OF L/240.	3 20 13.1 71.8 18.0 75.6 30.5 38.4
EXTERIOR CANOPIES/SOFFITS SEISMIC DATA 	60 PSF (STRENGTH)	BACKFILL AGAINST ANY STRUCTURAL ELEMENT UNTIL THAT ELEMENT HAS ATTAINED FULL DESIGN STRENGTH. DO NOT BACKFILL AGAINST BASEMENT WALLS UNTIL TOP AND BOTTOM OF WALL IS BRACED BY FLOOR FRAMING AND	FABRICATION, HANDLING, STORAGE, AND ERECTION SHALL BE IN ACCORDANCE WITH "TRUSS PLATE INSTITUTE"	3 50 11.9 52.1 15.7 68.7 29.4 36.1 3 100 11.1 37.2 14.0 63.4 28.5 34.3
SEISMIC IMPORTANCE FACTOR MAPPED SPECTRAL RESPONSE ACCELERATION FOR SHORT PERIODS (S MAPPED SPECTRAL RESPONSE ACCELERATION FOR 1 SECOND PERIOD	1.00 Ss) 0.050 (S1) 0.023	SLAB-ON-GRADE.	RECOMMENDED PRACTICES AND SHALL BE DONE IN A WORKMAN LIKE MANNER SO AS TO NOT DAMAGE THE TRUSSES. TRUSSES SHALL NOT BE CUT, ADDED ONTO, OR ALTERED IN ANY WAY WITH OUT THE WRITTEN	(-) WIND PRESSURE ON BOOF
SITE CLASS (ASSUMED) DESIGN SPECTRAL RESPONSE ACCELERATION FOR SHORT PERIODS (SE	D D Ds) 0.053	PROTECTION FROM FROST AND MINIMUM DEPTH TO SOILS CAPABLE OF PROVIDING DESIGN SOIL BEARING CAPACITY. UNCERTAINTIES INHERENT IN DETERMINING THE ELEVATION OF SOILS ADEQUATE TO PROVIDE DESIGN BEARING	CONSENT OF THE TRUSS DESIGNER, ENGINEER, AND ARCHITECT.	OVERHANGS
DESIGN SPECTRAL RESPONSE ACCELERATION FOR 1 SECOND PERIOD (SEISMIC DESIGN CATEGORY	(SD1) 0.037 A	CAPACITY MAY REQUIRE FOUNDATIONS TO BE LOWERED – IN NO CASE SHALL TOP OF FOOTING BE HIGHER THAN NOTED. A GEOTECHNICAL ENGINEER SHALL VERIFY THAT SOIL AT THE FOOTING BASE IS ADEQUATE TO PROVIDE THE	ENGINEER IN THE STATE OF FLORIDA FOR REVIEW BEFORE FABRICATION.	LOCATION WIND AREA (SF) 0° TO 7° 7° TO 27° 27° TO 45°
ANALYSIS PROCEDURE	MINIMUM EQUIVALENT LATERAL FORCE	 REQUIRED DESIGN SOIL BEARING CAPACITY. CAST-IN-PLACE CONCRETE 	WOOD TRUSS DESIGNER/SUPPLIER SHALL SUBMIT A SCHEDULE SHOWING TYPE, MANUFACTURER AND LOCATION FOR EACH TRUSS/ WALL CONNECTION AND TIE-DOWN ALONG WITH CAPACITY.	ZONE 2 ZONE 3 ZONE 2 ZONE 3 ZONE 2 ZONE 3
THE MATERIAL STRENGTHS AND STANDARDS LISTED HERE REPRESENT A SE NOTED IN THE SPECIFICATIONS. SEE SPECIFICATIONS FOR ADDITIONAL INFO	ELECTED SUMMARY OF THE REQUIREMENTS ORMATION. IN CASE OF DISCREPANCY	DESIGN AND CONSTRUCTION SHALL BE IN ACCORDANCE WITH THE PROVISIONS OF ACI 318 –11 EXCEPT WHERE MORE RESTRICTIVE REQUIREMENTS ARE NOTED.	CONTRACTOR SHALL SUBMIT TRUSS SHOP DRAWINGS FOR APPROVAL PRIOR TO FABRICATION. CONTRACTOR	OVERHANG 10 49.4 81.4 64.0 107.6 58.2 58.2 OVERHANG 20 48.6 63.9 64.0 97.1 56.4 56.4
BETWEEN THESE NOTES AND THE SPECIFICATIONS, THESE NOTES SHALL GO • SOILS	OVERN.	REINFORCING CLEAR COVER SHALL BE AS NOTED BELOW UNLESS SPECIFICALLY NOTED OTHERWISE ON STRUCTURAL	SHALL REVIEW AND STAMP ALL SHOP DRAWINGS BEFORE SUBMITTING TO THE ARCHITECT.	OVERHANG 50 47.4 40.8 64.0 83.2 54.1 54.1
 DESIGN SOIL BEARING CAPACITY FOR SPREAD/STRIP FOOTINGS CONCRETE (28 DAY STRENGTH) 	2,500 PSF	DRAWINGS. CONCRETE CAST AGAINST AND PERMANENTLY EXPOSED TO EARTH 3" CONCRETE EXPOSED TO EARTH OR WEATHER	SHOP DRAWINGS SUBMISSIONS SHALL INCLUDE THE FOLLOWING INFORMATION: NAME, ADDRESS, PHONE NUMBER OF TRUSS SUPPLIER SLOPE OF DEPTH, SPAN, AND SPACING	OVERHANG 100 46.5 23.3 64.0 72.7 52.4 52.4
FOOTINGS, DRILLED PIERS CONCRETE PIERS POSTJENSIONED SLAB	F'C = 3,000 PSI F'C = 4,000 PSI F'C = AS REQUIRED BY P.T. DESIGNER	#3 - #5 BARS 1 1/2" #6 - #18 BARS 2"	LOCATION OF ALL JOINTS ALL DESIGN LOADS	NOTES: 1) WIND PRESSURES PROVIDED IN TABLE ABOVE ARE BASED ON SIMPLIFIED PROVISIONS FOR ENCLOSED REGUL
REINFORCING STEEL	(3,000 PSI MINIMUM)	CONCRETE NOT EXPOSED TO EARTH OR WEATHER WALLS - #3 THRU #11 BARS 3/4"	ADJUSTMENTS TO LUMBER AND METAL CONNECTOR PLATE VALUES FOR CONDITIONS OF USE EACH REACTION FORCE AND DIRECTION	SHAPED BUILDINGS WITH MEAN ROOF HEIGHT LESS THAN OR EQUAL TO 60'-0" (ASCE 7-10)
WELDED WIRE FABRIC, PROVIDED IN FLAT SHEETS ONLY (ASTM A185) DEFORMED BARS (ASTM A615, GRADE 60)	Fy = 65,000 PSI Fy = 60,000 PSI	WALLS - #14 THRU #18 BARS 1 1/2" STRUCTURAL SLABS - TOP, BOTTOM 1"	METAL CONNECTOR PLATE TYPE, SIZE, GAUGE, AND DIMENSIONAL LOCATION OF EACH PLATE LUMBER SIZE, SPECIES, AND GRADE FOR EACH TRUSS MEMBER	 (+) = POSITIVE (INWARD) PRESSURE. (-) = NEGATIVE (OUTWARD) PRESSURE. SE = SOLIARE EFET
MASONRY SOLID CONCRETE BRICK (ASTM C55)	3,500 PSI	JOIST TIES AND MAIN REINFORCING - TOP, BOTTOM, SIDES 1 1/2" BEAM TIES - TOP, BOTTOM, SIDES 1 1/2" BEAM MAIN REINFORCING - TOP, BOTTOM, SIDES 2"	CONNECTION REQUIREMENTS FOR TRUSS TO TRUSS GIRDER, TRUSS PLY TO PLY, AND FIELD SPLICES CALCULATED DEFLECTION RATIO AND/OR MAXIMUM DEFLECTION FOR LIVE AND TOTAL LOADS SPECIEV ALL TRUSS TO TRUSS CONNECTIONS AND HANCERS	3) FOR EFFECTIVE MEMBER AREAS NOT SPECIFICALLY LISTED, INTERPOLATE OR USE LARGEST VALUE OF WIND F
CONCRETE MASONRY UNIT ASSEMBLY CONCRETE MASONRY UNIT (ASTM C90 - LIGHTWEIGHT)	F'm = 2,250 PSI 3,275 PSI	COLUMN MAIN REINFORCING - TOP, BOTTOM, SIDES 2 COLUMN TIES 1 1/2" COLUMN MAIN REINFORCING 2"	SPECIFY ALL TRUSS TO TRUSS CONNECTIONS AND HANGERS SPECIFY AND SHOW ALL PERMANENT TRUSS BRACING REQUIRED BY DESIGN	SUCTION NOTED. DO NOT USE 1/3 STRESS INCREASE FOR MEMBER DESIGN WITH VALUES NOTED IN THIS TABL
MORTAR (ASTM C270) GROUT (ASTM C476) ANCHOR RODS (ASTM E1554, GRADE 36)	F'c = 3,000 PSI Fy = 36,000 PSI	PROVIDE (2) #5 BARS AROUND ALL OPENINGS AND (2) #5 DIAGONAL BARS AT ALL OPENING AND RE-ENTRANT	CONTRACTOR IS RESPONSIBLE FOR ALL ERECTION PROCEDURES AND TEMPORARY TRUSS BRACE REQUIREMENTS DURING ERECTION IN ACCORDANCE WITH TPI'S COMMENTARY AND RECOMMENDATIONS FOR	4) FOR VALUES INDICATED BY THE FINAL VALUE, INCLUDING ALL PERMITTED REDUCTIONS, USED IN THE SHALL NOT BE LESS THAN A NET PRESSURE OF 16 PSF ACTING IN ANY DIRECTION NORMAL TO THE SURFACE.
 WOOD 		CORNERS. BARS SHALL EXTEND A MINIMUM OF 24" PAST OPENING.	HANDLING, INSTALL, AND BRACING METAL PLATE CONNECTED WOOD TRUSSES (HIP-91 BOOKLET) AND THE CURRENT EDITION OF ANSI/TPI-1.	5) LENGTH NOTED "a" = 3.0 FEET
WALL STUDS (SEE SCHEDULE) 2X SPF POSTS (MATCH WALL STUDS) 2X SPE JOIST/HEADERS/BEAMS		ALL BAR SPLICES SHALL BE CONTACT LAP SPLICED USING CLASS B TENSION LAP LENGTHS, WITH ADJACENT LAPS STAGGERED A MINIMUM OF 3'-0" UNLESS DETAILED OTHERWISE.	TRUSSES EXPOSED TO MOISTURE SHALL BE CONSTRUCTED OF PRESSURE TREATED WOOD AND GALVANIZED	
(SPF, NO.2 OR BETTER) Fb = 875 PSI Fv = 135 PS LAMINATED STRAND LUMBER (LSL) Fb = 2.300 PSI Fv = 310 PS	SI $F_{C} \perp = 425 \text{ PSI}$ E = 1,400,000 PSI SI $F_{C} \perp = 900 \text{ PSI}$ E = 1,500,000 PSI	FIELD WELDING OF ASTM A615 REINFORCING STEEL IS NOT PERMITTED. FIELD BENDING OF REINFORCING STEEL IS NOT PERMITTED EXCEPT WHERE SPECIFICALLY DETAILED ON STRUCTURAL DRAWINGS.	DESIGN ROOF TRUSSES TO RESIST ALL WIND LOADS INCLUDING UPLIFT AS REQUIRED BY THE FLORIDA BUILDING	$\begin{array}{c} 3 \\ 3 \\ 2 \\ 3 \\ 3 \\ 3 \\ 3 \\ 3 \\ 3 \\ 3 \\$
LAMINATED VENEER LUMBER (LVL) Fb = 3,100 PSI Fv = 285 PS PARALLEL STRAND LUMBER (PSL) Fb = 2,400 PSI Fv = 190 PSI	SI $Fc \perp = 750 \text{ PSI}$ $E = 2,000,000 \text{ PSI}$ I $Fc \parallel = 2,500 \text{ PSI}$ $E = 1,800,000 \text{ PSI}$	WOOD FRAMING	CODE. PROVIDE TIEDOWN CLIP AT EACH TRUSS AT EVERY POINT OF BEARING.	2 2 2 2 2 2 2
GLULAM BEAMS (SEE BEAM AND POST SCHED POSTS AND TIMBERS(SPF,NO.2 OR BETTER) Fb = 500 PSI	DULE) Fc II = 500 PSI E = 1,000,000 PSI	DESIGN AND CONSTRUCTION OF WOOD FRAMED CONSTRUCTION SHALL BE IN ACCORDANCE WITH THE PROVISIONS OF THE 2012 EDITION OF THE NATIONAL DESIGN SPECIFICATION (NDS) FOR WOOD CONSTRUCTION, INCLUDING THE 2012 EDITION OF THE NEED CONSTRUCTION, INCLUDING THE 2012 EDITION OF THE NEED CONSTRUCTION, AND THE CONSTRUCTION AND THE OPECAN.	ALL TRUSS TO TRUSS CONNECTIONS ARE TO BE DESIGNED, DETAILED, AND SUPPLIED BY THE TRUSS SUPPLIER.	
BOLTS AND LAG SCREWS (ASTM A307, GRADE A) RIM BOARDS WESTERN CEDAR (NO. 1) Fb = 2,400 PSI Fv = 285 PS	Fy = 36,000 PSI SI Fc \perp = 750 PSI E = 1,700,000 PSI SI Fc \perp = 425 PSI E = 1,000,000 PSI	THE NDS SUPPLEMENT DESIGN VALUES FOR WOOD CONSTRUCTION, AND THE 2008 EDITION OF THE SPECIAL DESIGN PROVISIONS FOR WIND AND SEISMIC STANDARD, EXCEPT WHERE MORE RESTRICTIVE REQUIREMENTS ARE NOTED.	TRUSS FABRICATOR SHALL FIELD VERIFY ALL SPAN DIMENSION BEFORE FABRICATING.	
WESTERN GEDAR (NO. 1) FD - 725 FSI FV - 155 FS	51 FC - 423 FSI E - 1,000,000 FSI	DESIGN AND CONSTRUCTION OF WOOD PANELS SHALL BE IN ACCORDANCE WITH THE PROVISIONS OF THE 2012 EDITION OF THE PANEL DESIGN SPECIFICATION. EXCEPT WHERE MORE RESTRICTIVE REQUIREMENTS ARE NOTED.	MECHANICAL SERVICES, ETC.	$\begin{array}{c c} & & & & \\ \hline & & & \\ \hline \\ \hline$
 CONSTRUCTION UNLESS SPECIFICALLY NOTED OTHERWISE, BUILDING STRUCTURE HAS BEEN 	N DESIGNED FOR THE FINAL COMPLETED	USE STEEL WASHERS BETWEEN HEAD AND NUT OF BOLT AND WOOD, AND BETWEEN HEAD OF LAG SCREW AND WOOD.	CONCRETE MASONRY DESIGN AND CONSTRUCTION SHALL BE IN ACCORDANCE WITH THE PROVISIONS OF ACI 530 - 11 AND ACI 530.1 -11	
CONDITION ONLY, AND HAS NOT BEEN ANALYZED, INVESTIGATED OR DESIGN INDIVIDUAL MEMBER, STABILITY DURING CONSTRUCTION. CONTRACTOR SH	NED FOR OVERALL STRUCTURE, OR ALL PROVIDE AND MAINTAIN TEMPORARY	WOOD SILL PLATES AND OTHER WOOD MEMBERS, INCLUDING PLYWOOD, DIRECTLY EXPOSED TO MOISTURE OR IN DIRECT	ALL CMU SHALL BE PLACED IN RUNNING BOND. UNLESS NOTED OTHERWISE PROVIDE CONTINUOUS LADDER TYPE	
BRACING AND SUPPORTS FOR ALL STRUCTURAL ELEMENTS, BOTH INDIVIDUA EVERY STAGE OF CONSTRUCTION UNTIL THE FINAL COMPLETION OF THE ST	ALLY AND COLLECTIVELY, AS REQUIRED AT RUCTURE. NO PORTION OF THE BUILDING	CONTACT WITH CONCRETE OR MASONRY SHALL BE PRESSURE TREATED.	REINFORCEMENT WITH 9 GAUGE SIDE AND CROSS RODS AT 16" OC VERTICALLY IN ALL WALLS AND PIERS, AND AT 8" OC VERTICALLY AT PARAPETS. WHERE VERTICAL BARS ARE REQUIRED, CONSTRUCT CMU WALL TO PROVIDE A	FLAT ROOF GABLE ROOF ROOF SLOPE 1/2" ROOF SLOPE 7°< L <45°
TEMPORARY BRACES AND SUPPORTS, WHICH SHALL ADDITIONALLY PROVIDE	E SUPPORT FOR ALL CONSTRUCTION	NOT BE PLACED LESS THAN 3/8" FROM PANEL EDGE.	CONTINUOUS UNOBSTRUCTED CELL FROM BOTTOM TO TOP OF BAR. CELL CONTAINING A SINGLE BAR SHALL NOT BE LESS THAN 3" X 4" IN PLAN AREA.	
NOT EXCEED THE DESIGN FLOOR LOADING.		MAXIMUM MOISTURE CONTENT IN ANY WOOD MEMBER SHALL NOT EXCEED 19%.	PORTIONS OF CMU CONSTRUCTION REQUIRING STRUCTURAL FILL SHALL USE GROUT ONLY. USE OF CONCRETE FILL	2 1 2 3 3 3 3 3 3 3 3 3 3
CONTRACTOR IS SOLELY RESPONSIBLE FOR THE MEANS, METHODS, TECHNI CONSTRUCTION INCLUDING, BUT NOT LIMITED TO, TEMPORARY BRACING, SU	IQUES, SEQUENCES AND PROCEDURES OF JPPORTS, SHORING, FORMING TO	THE CONTRACTOR MAY CHOOSE TO UTILIZE A CONTINUOUS LOAD PATH (CLP) THREADED ROD HOLD-DOWN SYSTEM IN LIEU OF STRAPS AND HOLD-DOWN ANCHORS AT SHEAR WALL LOCATIONS. IF A CLP SYSTEM IS UTILIZED, THE CONTRACTOR SHALL SUBMIT	WITH PEA GRAVEL AGGREGATE; OTHERWISE USE FINE GROUT.	
SUPPORT IMPOSED CONSTRUCTION LOADS, AND OTHER SIMILAR ITEMS.		FOR REVIEW CALCULATIONS AND SHOP DRAWINGS SEALED BY A PROFESSIONAL ENGINEER REGISTERED IN THE STATE OF FLORIDA.	REFER TO ARCHITECTURAL DRAWINGS FOR LOCATION OF ALL VERTICAL CONTROL JOINTS IN EXTERIOR WYTHES OF PERIMETER WALLS AND FOR EXTERIOR WALLS.	
INTENDED TO IDENTIFY ALL APPLICABLE OSHA REQUIREMENTS.	I ENERGES ANT INCIDENTAL, AND ARE NUT	STRUCTURAL DRAWINGS. THE ZIP SYSTEM WALL SHEATHING SHALL COMPLY WITH DOC PS 2 FOR WOOD STRUCTURAL PANELS AND ICC-ES EVALUATION REPORT ESR-1474 (CURRENT ISSUE). REFER TO SHEAR WALL SCHEDULE FOR REQUIRED THICKNESS AND	PROVIDE STEEL PIPE SLEEVES AT ALL LOCATIONS WHERE PIPING PASSES THROUGH CMU WALL.	$\begin{array}{ c c c c c c c c c c c c c c c c c c c$
INFORMATION CONTAINED IN THE GENERAL NOTES IS ONLY A PARTIAL SUMN SEE SPECIFICATIONS, PLANS AND DETAILS FOR ADDITIONAL REQUIREMENTS	MARY OF PROJECT REQUIREMENTS. S.	FASTENING PATTERN.	WHERE BOND BEAMS INTERSECT AT WALL CORNERS AT DIFFERENT ELEVATIONS, RUN EACH BOND BEAM AROUND THE	
USE ONLY DIMENSIONS INDICATED ON THE DRAWINGS. DO NOT MANUALLY S	SCALE THE DRAWINGS OR USE ANY	 POST-TENSIONED CONCRETE DESIGN AND CONSTRUCTION SHALL BE IN ACCORDANCE WITH THE PROVISIONS OF ACI 318-11 EXCEPT WHERE MORE RESTRICTIVE REQUIREMENTS ARE NOTED 	SAME WALL AT DIFFERENT ELEVATIONS, RUN BOND BEAMS PAST ONE ANOTHER A MINIMUM OF FOUR FULL BLOCK LENGTHS BEFORE TERMINATING	$\begin{array}{c} \times & \times \\ & & \\ & \\ & \\ & \\ & \\ & \\ & \\ &$
DIMENSIONS MEASURED FROM ELECTRONIC DRAWING FILES.		SHOP DRAWINGS: SHOP DRAWINGS AND DESIGN CALCULATIONS, BEARING STAMP AND SIGNATURE OF PROFESSIONAL ENGINEER		ROOF SLOPE 0°< L <7° ROOF SLOPE 1°< L <27°
CENTERLINES, AND FRAMING ELEMENTS ARE EQUALLY SPACED BETWEEN AI	DJACENT COLUMN CENTERLINES.	REGISTERED IN STATE OF FLORIDA, SHOWING COMPLETE INFORMATION FOR INSTALLATION OF POST TENSIONED CONCRETE. INDICATING DIMENSIONS AND LOCATIONS AS WELL AS SIZE AND TYPE OF REINFORCEMENT. INDICATE LAYOUT, DIMENSIONS, AND DESCRETURE OF INSTALLATION, DESCRETE LOCATION, AND DETAILS OF ANOLISIAS AND LOCATION AND DETAILS		
MAJOR OPENING LOCATIONS AND SIZES ARE INDICATED ON THE STRUCTURA AND SLEEVES REQUIRED TO ACCOMMODATE VARIOUS BUILDING SERVICES M	AL DRAWINGS - SMALLER OPENINGS MAY NOT BE NOTED. CONTRACTOR TO	DIMENSIONS, AND PROCEDURE OF INSTALLATION. PROVIDE LOCATION AND DETAILS OF ANCHORAGE DEVICES THAT ARE TO BE EMBEDDED.		
VERIFY THE SIZE AND LOCATION OF ALL ARCHITECTURAL, MECHANICAL, ELE INCLUDING CLEARANCE REQUIREMENTS CONTAINED IN THE RESPECTIVE DIS	ECTRICAL AND PLUMBING OPENINGS, SCIPLINE DOCUMENTS FOR INSTALLATION	MIX DESIGN FOR POST-TENSIONED CONCRETE IS BY PT FOUNDATION DESIGNER.	AB ANCHOR BOLT (ROD) EW EACH WAY AHU AIR HANDLING UNIT EWEE EACH WAY EACH FACE I P LOW POINT	RD ROOF DRAIN REF REFERENCE
PENETRATIONS BE MADE IN ANY STRUCTURAL ELEMENT AFTER FINAL PLACE WITHOUT WRITTEN APPROVAL OF THE STRUCTURAL ENGINEER.	EMENT IN THE BUILDING STRUCTURE,	FUTURE PENETRATIONS OF POST-TENSIONED SLABS REQUIRE CAREFUL ADHERENCE TO THE FOLLOWING PROCEDURE: OBTAIN THE SIZE AND DESIRED LOCATION OF THE NEW OPENING.	ALT ALTERNATE EXP EXPANSION LSL LAMINATED STRAND LUME APPROX APPROXIMATELY EXT EXTERIOR LTWT LIGHTWEIGHT	ER REINF REINFORCE(D) REM REMAINDER
CONSULT ARCHITECTURAL, MECHANICAL, ELECTRICAL AND PLUMBING DRAW	VINGS AND MANUFACTURERS SPEC	REVIEW STRUCTURAL DESIGN CALCULATIONS FOR IMPACT OF OPENING. IF NEW OPENING IS ACCEPTABLE IN THE DESIRED LOCATION, LOCATE TENDONS AND MILD REINFORCEMENT USING EQUIPMENT DESIGNED FOR THE DURDONS (DACHOMETER FOURMENT)	ARCH ARCHITECTURAL EXTG EXISTING LVL LAMINATED VENEER LUME B/F BOTTOM OF FOOTING FD FLOOR DRAIN LW LONG WAY	ER RTU ROOF TOP UNIT SC SLIP CRITICAL
SHEETS FOR LOCATIONS AND DIMENSIONS OF PADS, CURBS, EQUIPMENT SU REGLETS, REVEALS, FINISHES AND OTHER MISCELLANEOUS PROJECT REQU	JPPORTS, DEPRESSIONS, INSERTS, DRIPS, IIREMENTS THAT NECESSITATE	ADJUST OPENING LOCATION TO AVOID CUTTING ANY TENDONS AND TO MINIMIZE CUTTING OF MILD REINFORCEMENT.	B/S BOTTOM OF STEEL FLG FLANGE MAX MAXIMUM BC BOTTOM CHORD FLR FLOOR MECH MECHANICAL BLDG BUILDING FTG FOOTING MER MANUFACTURER	SCH SCHEDULE SHT SHEET SIM SIMILAR
INCIDENTAL ACCOMMODATION BY THE BUILDING STRUCTURE BUT ARE NOT S GENERAL THE STRUCTURE HAS BEEN DESIGNED AS UNDESTRAINED FOR THE DUPDOS		REINFORCEMENT. WORK MUST BE SUPERVISED BY AN INDIVIDUAL COMPETENT TO DETERMINE THAT THE STRUCTURAL SYSTEM IS NOT DAMAGED	BRGBEARINGFUTFUT FUTUREMINMINIMUMBTWNBETWEENFVFIELD VERIFYMISCMISCELLANEOUS	SL SNOW LOAD SLBB SHORT LEGS BACK TO BACK
ASSEMBLY EVALUATIONS.	DE OF FIRE RATING AND FIREPROUPING		CB CATCH BASIN GA GAUGE MO MASONRY OPENING CIP CAST-IN-PLACE GALV GALVANIZED MS MIDDLE STRIP	SOG SLAB-ON-GRADE SP SPAC(ES)(ED)(ING) STRUCTURAL SHEET INDEX
STRUCTURAL COMPONENTS HAVE NOT BEEN DESIGNED FOR VIBRATORY EQ PLACE VIBRATORY EQUIPMENT AND EQUIPMENT SENSITIVE TO VIBRATIONS	QUIPMENT UNLESS NOTED OTHERWISE. ON VIBRATION ISOLATORS SPECIFICALLY	EMBEDMENT DEPTH OF POST-TENSIONED SLAB SHOWN ON THESE DRAWINGS SHALL BE REVIEWED, VERIFIED AND APPROVED BY PROJECT GEOTECHNICAL ENGINEER.	CL CENTER LINE GLULAM GLUE-LAMINATED BEAM(S) NIC NOT IN CONTRACT CL CENTER LINE GLULAM GLUE-LAMINATED BEAM(S) NIC NOT IN CONTRACT	SPEC SPECIFICATION(S) SQ SQUARE SS STAINUESS STEEL
			CMU CONCRETE MASONRY UNIT HK HOOK NTS NOT TO SCALE COL COLUMN HORIZ HORIZONTAI OC ON CENTER	STAINLESS STELL S.I.11 PLANS AND SCHEDULES - TRASH ENCL STD STANDARD S.T.80 FOUNDATION DETAILS - TRASH ENCL SW SHORT WAY STANDARD
LATERAL BRACING FOR NON-STRUCTURAL ELEMENTS DESIGNED AND DETAIL DESIGNED TO APPLY LOADS DIRECTLY TO FLOOR OR ROOF DIAPHRAGMS. B BOTTOM FLANGES OF BEAMS OR BOTTOM CHORDS OF JOISTS UNLESS THE	LED BT GUMPONENT SUPPLIERS SHALL BE BRACES SHALL NOT ATTACH DIRECTLY TO COMPONENT SUPPLIER PROVIDES		CONCCONCRETEHPHIGH POINTODOUTSIDE DIAMETERCONTCONTINUOUSHVACHEATING, VENTILATING,OFOUTSIDE FACE	T/FTOP OF FOOTINGS.T.90STRUCTURAL DETAILS - TRASH ENCLT/LTOP OF LEDGE
ADDITIONAL BRACING FROM THOSE ELEMENTS TO THE FLOOR OR ROOF DIA	PHRAGM AT EACH ATTACHMENT POINT.		CS COLUMN STRIP AND AIR CONDITIONING OPING OPENING DBA DEFORMED BAR ANCHOR HWS HEADED WELDED STUD(S) OPP OPPOSITE DEMO DEMOLISIER ID INCIDE DIAMETER	T/P TOP OF PIER T/S TOP OF STEEL
HOLES, NOTCHES, BLOCK-OUTS AND OTHER SIMILAR FIELD MODIFICATIONS SPECIFICALLY DETAILED ON THE STRUCTURAL DRAWINGS OR APPROVED SH	TO STRUCTURAL MEMBERS NOT HOP DRAWINGS ARE NOT PERMITTED.		DEIVICE DEIVICEI IN INSIDE DIAMETER OSL OUTSTANDING LEG DIA DIAMETER IF INSIDE FACE PC PRECAST / PRESTRESSED DL DEAD LOAD INT INTERIOR DCL DOLINDS DED CLUDIC INCL	TOP OF WALL TC TENSION CONTROL TC TOP CHORD
EXCEPT AS NOTED BELOW, ALL FUTURE EXPANSION IS ASSUMED TO BE COM	MPLETELY SELF SUPPORTING FOR BOTH		DWG DRAWING JBE JOIST BEARING ELEVATION PDF POUNDS PER CUBIC FOOT E/D EDGE OF DECK K KIP PI PI ATF	THK THICK (NESS) (ENED) TL TOTAL LOAD
GRAVITY AND LATERAL LOADS.			E/S EDGE OF SLAB KO KNOCKOUT PANFI PI BG PI UMBING	TYP TYPICAL THRU-OUT THE

EDGE OF DECK E/D E/S EDGE OF SLAB EACH FACE EF EXPANSION JOINT EJ ELEVATION EL ELEC ELECTRICAL ENG EQ ENGINEER EQUAL

EDGE STRIP

ES

DD) T	EW EWEF EXP EXT	EACH WAY EACH WAY EACH FACE EXPANSION EXTERIOR
ING -	EXTG FD FLG FLR FTG FUT FV	EXISTING FLOOR DRAIN FLANGE FLOOR FOOTING FUTURE FIELD VERIFY
	GA GALV GC GLULAM	GAUGE GALVANIZED GENERAL CONTRACTO GLUE-LAMINATED BEA
) NRY UNIT	gt HK Horiz HP HVAC	GIRDER TRUSS HOOK HORIZONTAL HIGH POINT HEATING, VENTILATING AND AIR CONDITION
NCHOR IOLISH	HWS ID IF INT JBE K	HEADED WELDED STU INSIDE DIAMETER INSIDE FACE INTERIOR JOIST BEARING ELEVA KIP
	KO KSI LB LL LLBB LLH LLV	KNOCKOUT PANEL KIPS PER SQUARE INC ANGLE POUNDS LIVE LOAD LONG LEG BACK TO BA LONG LEG HORIZONTA LONG LEG VERTICAL

EACH FACE N	LP LSL	LOW POINT LAMINATED STRAND LUMBER
ΔΙΝ		LIGHTWEIGHT LAMINATED VENEER LUMBER
	MAX	MAXIMUM
	MECH	MECHANICAL
	MFR	MANUFACTURER
	MIN	MINIMUM
IFY	MISC	MISCELLANEOUS
-0	MO	
	IVIS NIA	
	NOM	NOMINAL
000	NTS	NOT TO SCALE
AI	00	ON CENTER
T	OD	OUTSIDE DIAMETER
/ENTILATING,	OF	OUTSIDE FACE
CONDITIONING	OPNG	OPENING
ELDED STUD(S)	OPP	OPPOSITE
METER	OSL	OUTSTANDING LEG
)Е	PC	PRECAST / PRESTRESSED
	PCI	POUNDS PER CUBIC INCH
RING ELEVATION	PDF PL	POUNDS PER CUBIC FOOT PLATE
Γ PANEL	PLBG	PLUMBING
SQUARE INCH	PLF	POUNDS PER LINEAR FOOT
	PROJ	PROJECTION
	PSF	POUNDS PER CUBIC FOOT
	PSI	POUNDS PER SQUARE INCH
BACK TO BACK HORIZONTAL	PT	PRE (POST) -TENSIONED

OMPONENTS AND CLADDING ASD WIND PRESSURES (PSF)											
				ROOF	SLOPE						
NE WIND AREA (SF)	0° TO 7°		7° TO 27°		27° TO 45°		ZONE	WIND AREA (SE)			
	/	(+)	(-)	(+)	(-)	(+)	(-)		/	(+)	(-)
1	10	14.0	34.3	19.8	31.4	31.4	34.3	4	10	31.4	34.0
1	20	13.1	33.4	18.0	30.5	30.5	32.6	4	20	30.0	32.6
1	50	11.9	32.3	15.7	29.4	29.4	30.3	4	50	28.2	30.8
1	100	11.1	31.4	14.0	28.5	28.5	28.5	4	100	26.8	29.4
2	10	14.0	57.6	19.8	54.7	31.4	40.1	5	10	31.4	41.9
2	20	13.1	51.5	18.0	50.3	30.5	38.4	5	20	30.0	39.1
2	50	11.9	43.4	15.7	44.5	29.4	36.1	5	50	28.2	35.4
2	100	11.1	37.2	14.0	40.1	28.5	34.3	5	100	26.8	32.6
3	10	14.0	86.7	19.8	80.9	31.4	40.1				
3	20	13.1	71.8	18.0	75.6	30.5	38.4				
3	50	11.9	52.1	15.7	68.7	29.4	36.1				
3	100	11.1	37.2	14.0	63.4	28.5	34.3				

(-) WIND PRESSURE ON ROOF OVERHANGS

	WIND	ROOF SLOPE							
UN	AREA (SF)	0° TO 7°		7° TO 27°		27° TO 45°			
		ZONE 2	ZONE 3	ZONE 2	ZONE 3	ZONE 2	ZONE 3		
NG	10	49.4	81.4	64.0	107.6	58.2	58.2		
NG	20	48.6	63.9	64.0	97.1	56.4	56.4		
NG	50	47.4	40.8	64.0	83.2	54.1	54.1		
NG	100	46.5	23.3	64.0	72.7	52.4	52.4		

EFFECTIVE MEMBER AREAS NOT SPECIFICALLY LISTED, INTERPOLATE OR USE LARGEST VALUE OF WIND PRESSURE TION NOTED. DO NOT USE 1/3 STRESS INCREASE FOR MEMBER DESIGN WITH VALUES NOTED IN THIS TABLE. VALUES INDICATED BY THE FINAL VALUE, INCLUDING ALL PERMITTED REDUCTIONS, USED IN THE DESIGN



RD	
RFF	REFERENCE
REINF	REINFORCE(D)
REM	REMAINDER
RTU	ROOF TOP UNIT
SC	SLIP CRITICAL
SCH	SCHEDULE
SHT	SHEET
SIM	SIMILAR
SL	SNOW LOAD
SLBB	SHORT LEGS BACK TO BACK
SOG	SLAB-ON-GRADE
SP	SPAC(ES)(ED)(ING)
SPEC	SPECIFICATION(S)
SQ	SQUARE
SS	STAINLESS STEEL
SID	STANDARD
SW	
T/L T/D	
1/P T/S	
T/M	
TC	
TC	TOP CHORD
THK	THICK (NESS) (ENED)
TL	TOTAL LOAD
TYP	TYPICAL THRU-OUT THE
	CONSTRUCTION DOCUMENTS
UNO	UNLESS NOTED OTHERWISE
VERT	VERTICAL
VIF	VERIFY IN FIELD
WL	WIND LOAD
WP	WORKING POINT
WWF	WELDED WIRE FABRIC

STRUCTURAL SHEET INDEX

- S.0 STRUCTURAL NOTES TRASH ENCLOSURE
- S.T.11 PLANS AND SCHEDULES TRASH ENCLOSURE S.T.80 FOUNDATION DETAILS - TRASH ENCLOSURE
- S.T.90 STRUCTURAL DETAILS TRASH ENCLOSURE

Kahler Slater experience design

CONTINENTAL PROPERTIES CONTINENTAL 422 FUND LLC

W134 N8675 EXECUTIVE PARKWAY MENOMONEE FALLS, WI 53051 262.502.5500 * FAX 262.502.5522

rasmith 16745 W. Bluemound Brookfield, WI 5300.5 (262) 781-1000 CREATIVITY BEYOND ENGINEERING rasmith.com

16745 W. Bluemound Road Brookfield, WI 53005-5938

project number: 1160189 Contractors are responsible for the means, methods, techniques, sequences and procedures of construction including, but not limited to, temporary supports, shoring, forming to support imposed loads and other similar items.



Revisions

BID/PERMIT SET

Drawing Date

JANUARY 31, 2018

TRASH ENCLOSURE

SPRINGS AT HAMMOCK COVE THOMASSON DR. & CARDINAL WAY NAPLES, FL 34112

Project No. 216026.04

Sheet Title STRUCTURAL NOTES -TRASH ENCLOSURE

Copyright © 2018 Kahler Slater, Inc. All rights reserved.

111 West Wisconsin Avenue, Milwaukee, Wisconsin 53203 Telephone 414.272.2000 Fax 414.272.2001

44 East Mifflin Street, Suite 700, Madison, Wisconsin 53703 Telephone 608.283.6300 Fax 608.283.6317 Sheet No.

S.0



- PLATES (ASTM A36) STRUCTURAL STEEL (CONNECTIONS) WELDING ELECTRODES
- GROUT (ASTM C1107)

E70XX F'c = 5,000 PSI

CONTINENTAL PROPERTIES

CONTINENTAL 422 FUND LLC W134 N8675 EXECUTIVE PARKWAY MENOMONEE FALLS, WI 53051 262.502.5500 * FAX 262.502.5522

Contractors are responsible for the means, methods, techniques, sequences and procedures of construction including, but not limited to, temporary supports, shoring, forming to support imposed loads and other similar items.

raSmith 16/45 W. Bluemound Brookfield, WI 5300 (262) 781-1000 CREATIVITY BEYOND ENGINEERING rasmith.com

project number: 1160189

16745 W. Bluemound Road Brookfield, WI 53005-5938

CONCRETE PIER SCHEDULE
 PIER DIMENSIONS
 PIER
 REINFORCE

 X
 Y
 TYPE
 VERTICAL
 REINFORCEMENT TIES REMARKS (8) #5 #3 AT 12" OC. TOP OF SLAB IS EQUAL TO 16" TOP OF PIER #3 AT 12" OC. REFER TO CIVIL PLANS (8) #5 16" FOR ELEVATIONS

1. REFERENCE DETAIL 1/S.T.11 FOR TYPICAL PIER INFORMATION. PROVIDE 2" CLEAR COVER AT ALL PIER TYPES



FOUNDATION PLAN NOTES

1. SEE STRUCTURAL NOTES FOR ADDITIONAL FOUNDATION REQUIREMENTS. 2. ELEVATION 100'-0" ON STRUCTURAL DRAWINGS CORRESPONDS TO FINISHED FLOOR ELEVATION SHOWN ON SITE PLAN. TOP OF SLAB ELEVATION VARIES. REFER TO ARCHITECTURAL DRAWINGS FOR SLAB PITCH.

3. FOUNDATIONS SHALL BE POST TENSIONED SLAB DESIGNED BY OTHERS. SEE S.0, S.T.80, AND S.T.90 FOR ADDITIONAL DETAILS. ALL CONCRETE PIERS SHALL BE MILD REINFORCED. REFER TO GEOTECH REPORT FOR BASE 4. PROVIDE SOIL TREATMENT FOR TERMITE PROTECTION PER THE

REQUIREMENTS OF THE 2017 FLORIDA BUILDING CODE SECTION 1816. 5. OVER-EXCAVATION PER DETAIL 2/S.T.90 MAY BE REQUIREDTO REMOVE EXISTING UNDOCUMENTED FILL AND UNSUITABLE BEARING SOIL. 6. ADDITIONAL DETAILS THAT MAY APPLY BUT, ARE NOT SPECIFICALLY

1/S.T.90 MASONRY BOND BEAM CORNER REINFORCEMENT PLAN DETAIL

FOUNDATION KEY NOTES

(1) 8"X8" MASONRY PIER WITH (1) #5 VERTICAL GROUTED SOLID FULL-HEIGHT. (2) NOTE: CONCRETE PIERS ARE NOT POST TENSIONED.

NOTE: ALL LOADS ARE SERVICE LEVEL

FOUNDATION LEGEND

CONCRETE PAD FOOTING)
)
CONCRETE PIER	
COLUMN FOOTING MARKF40 TOP OF COLUMN FOOTING ELEVATION99'-0" CONCRETE PIER MARKP1 TOP OF PIER ELEVATION100'-0"	
CONCRETE WALL AND FOOTING	
TOP OF WALL FOOTING ELEVATION96'-0"	
TOP OF LEDGE ELEVATIONT/L=99'-6"	
TOP OF WALL ELEVATION T/W=100'-0"	
	٦
F40 F40	
99'-0" └	ļ
100'-0"	İ
WALL FOOTING STEP MARKER]
SLAB-ON-GRADE JOINT	
TOP OF EXISTING WALL FOOTING ELEVATION - 96'-0"	



Revisions

BID/PERMIT SET

Drawing Date JANUARY 31, 2018

TRASH ENCLOSURE

SPRINGS AT HAMMOCK COVE THOMASSON DR. & CARDINAL WAY NAPLES, FL 34112

Project No. 216026.04

Sheet Title PLANS AND SCHEDULES - TRASH ENCLOSURE

Copyright © 2018 Kahler Slater, Inc. All rights reserved.

111 West Wisconsin Avenue, Milwaukee, Wisconsin 53203 Telephone 414.272.2000 Fax 414.272.2001

44 East Mifflin Street, Suite 700, Madison, Wisconsin 53703 Telephone 608.283.6300 Fax 608.283.6317 Sheet No.

S.T.11





CONTINENTAL PROPERTIES CONTINENTAL 422 FUND LLC

W134 N8675 EXECUTIVE PARKWAY MENOMONEE FALLS, WI 53051 262.502.5500 * FAX 262.502.5522

raSmith CREATIVITY BEYOND ENGINEERING rasmith.com

16745 W. Bluemound Road Brookfield, WI 53005-5938 (262) 781-1000

project number: 1160189 Contractors are responsible for the means, methods, techniques, sequences and procedures of construction including, but not limited to, temporary supports, shoring, forming to support imposed loads and other similar items.



Revisions

BID/PERMIT SET

Drawing Date JANUARY 31, 2018

TRASH ENCLOSURE

SPRINGS AT HAMMOCK COVE THOMASSON DR. & CARDINAL WAY NAPLES, FL 34112

Project No. 216026.04

Sheet Title FOUNDATION DETAILS - TRASH ENCLOSURE

Copyright © 2018 Kahler Slater, Inc. All rights reserved. 111 West Wisconsin Avenue, Milwaukee, Wisconsin 53203 Telephone 414.272.2000 Fax 414.272.2001

44 East Mifflin Street, Suite 700, Madison, Wisconsin 53703 Telephone 608.283.6300 Fax 608.283.6317

Sheet No. S.T.80





CONTINENTAL PROPERTIES CONTINENTAL 422 FUND LLC

W134 N8675 EXECUTIVE PARKWAY MENOMONEE FALLS, WI 53051 262.502.5500 * FAX 262.502.5522

raSmith CREATIVITY BEYOND ENGINEERING rasmith.com

16745 W. Bluemound Road Brookfield, WI 53005-5938 (262) 781-1000

project number: 1160189 Contractors are responsible for the means, methods, techniques, sequences and procedures of construction including, but not limited to, temporary supports, shoring, forming to support imposed loads and other similar items.



Revisions

BID/PERMIT SET

Drawing Date JANUARY 31, 2018

TRASH ENCLOSURE

SPRINGS AT HAMMOCK COVE THOMASSON DR. & CARDINAL WAY NAPLES, FL 34112

Project No. 216026.04

Sheet Title STRUCTURAL **DETAILS - TRASH** ENCLOSURE

Copyright © 2018 Kahler Slater, Inc. All rights reserved. 111 West Wisconsin Avenue, Milwaukee, Wisconsin 53203 Telephone 414.272.2000 Fax 414.272.2001

44 East Mifflin Street, Suite 700, Madison, Wisconsin 53703 Telephone 608.283.6300 Fax 608.283.6317

Sheet No. S.T.90