STRUCTURAL GENERAL NOTES:

DESIGN AND LOADING
1. THE STRUCTURAL DESIGN OF THIS BUILDING WAS BASED ON THE DESIGN CRITERIA:
   A. BUILDING CODE: FLORIDA BUIILDING CODE 2014
   B. FLOOR:
      LIVE LOAD: 100 PSF
   C. ROOF:
      LIVE LOAD: 20 PSF
      DEAD LOAD: 25 PSF (INCLUDES 5 PSF FOR SPRINKLERS)
      SPRINKLER LIVE LOAD: 250 LBS
   D. WIND:
      BASIC WIND SPEED: 132 MPH (3-SECOND GUST)
      ULTIMATE WIND SPEED: 170 MPH (3 SECOND GUST)
      BUILDING OCCUPANCY CATEGORY: II
      WIND EXPOSURE: D
      PRESSURES PER ASCE7-10
   E. FLOOD LOAD: N/A
   F. SPECIAL LOADS: N/A
   G. CANOPY: N/A

FOUNDATION NOTES
1. THE FOUNDATION DESIGN OF THIS BUILDING WAS BASED ON THE FOLLOWING CRITERIA:
   A. ALLOWABLE SOIL BEARING CAPACITY= 2,500 psf
   B. ANY FILL REQUIRED BELOW SLABS ON GRADE OR FOOTINGS SHALL BE COMPACTED.
2. ALL EXTERIOR FOOTINGS SHALL EXTEND BELOW THE MAXIMUM ANTICIPATED DEPTH OF FROST.
3. THE CONTRACTOR SHALL NOTIFY THE ARCHITECT OR ENGINEER OF RECORD IMMEDIATELY IN THE EVENT THAT THE SOILS CONDITIONS ENCOUNTERED VARY FROM THOSE SHOWN ON THE BORING LOGS.
4. ALL FOUNDATION EXCAVATIONS SHALL BE INSPECTED BY A SOILS TESTING LABORATORY PRIOR TO PLACEMENT OF CONCRETE.

CONCRETE AND REINFORCING
1. ALL CONCRETE SHALL BE IN ACCORDANCE WITH THE "AMERICAN CONCRETE INSTITUTE BUILDING CODE" (ACI 318) AND WITH "SPECIFICATIONS FOR STRUCTURAL CONCRETE FOR BUILDINGS" (ACI 301) LATEST EDITIONS.
2. ALL NORMAL WEIGHT CONCRETE (145 PCF) SHALL OBTAIN A MINIMUM 28 DAY COMPRESSIVE STRENGTH OF 3000 PSI (3000 PSI FOR SLABS).
3. ALL CONCRETE SUBJECT TO EXTERIOR EXPOSURE SHALL BE AIR ENTRAINED AS RECOMMENDED BY ACI 318.
4. TEST CYLINDERS SHALL BE MADE AND TESTED AS OUTLINED IN CHAPTER 16 OF ACI-301.
5. REINFORCING BARS SHALL BE DEFORMED BARS OF NEW BILLET STEEL CONFORMING TO ASTM A-615, GRADE 60. WELDED WIRE FABRIC SHALL CONFORM TO ASTM A-185. ALL REINFORCING AND ACCESSORIES SHALL BE DETAILED AND PLACED IN ACCORDANCE WITH ACI STANDARD 315 AND 315R.
6. PROVIDE ALL ACCESSORIES NECESSARY TO SUPPORT REINFORCEMENT AT POSITIONS SHOWN ON THE PLANS AND DETAILS. PLASTIC COATED ACCESSORIES SHALL BE USED IN ALL EXPOSED CONCRETE WORK.
7. THE GENERAL CONTRACTOR SHALL CHECK WITH ARCHITECTURAL, MECHANICAL AND ELECTRICAL DRAWINGS AND THE SUB-CONTRACTORS FOR OPENINGS, SLEEVES, ANCHORS, HANGERS, INSERTS, SLAB DEPRESSIONS AND OTHER ITEMS RELATED TO THE CONCRETE WORK AND SHALL ASSUME RESPONSIBILITY FOR THEIR PROPER LOCATION.

STRUCTURAL STEEL
1. STRUCTURAL STEEL SHALL BE DETAILED, FABRICATED AND ERECTED IN ACCORDANCE WITH THE AISC "STEEL CONSTRUCTION MANUAL", 14TH EDITION.
2. STRUCTURAL STEEL SHALL CONFORM TO THE FOLLOWING ASTM SPECIFICATIONS:
   A. ANCHOR RODS — F1554, GRADE 36
   B. HIGH STRENGTH STRUCTURAL BOLTS — A325-N U.N.O.
   C. STRUCTURAL SHAPES (W) — A992
   D. STRUCTURAL SHAPES (M, S, C, MC, PLATES) — A36
   E. STRUCTURAL SHAPES (HP) — A572
   F. STRUCTURAL TUBING (HSS) — A500 GRADE B
   G. STRUCTURAL ANGLES — A36
3. ALL WELDING ELECTRODES SHALL BE E70-XX. ALL SHOP AND FIELD WELDING SHALL BE MADE IN ACCORDANCE WITH A.W.S. D1.1 "CODE FOR WELDING IN BUILDING CONSTRUCTION" AND SHALL BE MADE BY CERTIFIED WELDERS.

LIGHT GAGE METAL FRAMING
1. 16 GA. AND HEAVIER STUDS SHALL HAVE A MINIMUM YIELD STRESS OF 50,000 PSI.
   18 GA. AND LIGHTER STUDS AND TRACKS SHALL HAVE A MINIMUM YIELD STRESS OF 33,000 PSI.
2. STUDS AND TRACKS SHALL BE 18 GA. MINIMUM U.N.O. THEY SHALL BE MANUFACTURED BY DIETRICH INDUSTRIES, INC. OR APPROVED EQUAL.
3. PROVIDE DOUBLE STUDS FOR FULL HEIGHT OF WALL EACH SIDE OF ALL OPENINGS UNLESS OTHERWISE NOTED. WELD STUDS TO EACH OTHER WITH 1 1/2" LONG 1/8" FILLET WELDS AT 12" O.C. EACH SIDE. PROVIDE STUD TRACK AT EACH HEAD AND SILL.
4. REFER TO PLANS AND DETAILS FOR CONNECTION OF STUD WALLS TO FOUNDATION, FLOOR OR ROOF.

STEEL JOISTS
1. STEEL JOISTS SHALL BE DESIGNED, FABRICATED AND ERECTED IN ACCORDANCE WITH THE STEEL JOIST INSTITUTE SPECIFICATIONS. PROVIDE ALL ACCESSORIES NECESSARY FOR COMPLETE INSTALLATION OF ALL STEEL JOISTS, INCLUDING BRIDGING, AS REQUIRED BY THE DRAWINGS AND THE STEEL JOIST INSTITUTE SPECIFICATIONS.

METAL DECK
1. ALL METAL DECK SHALL BE DETAILED, FABRICATED, AND INSTALLED IN ACCORDANCE WITH THE STEEL DECK INSTITUTE SPECIFICATIONS, LATEST EDITION.
2. ALL METAL DECK SHALL BE CONTINUOUS OVER THREE OR MORE SPANS, EXCEPT WHERE STEEL LAYOUT DOES NOT PERMIT.
3. METAL ROOF DECK SHALL BE 20 GAUGE, 1 1/2" DEEP, TYPE B, 36/7 FASTENER LAYOUT WIDE RIB METAL DECK, GALVANIZED.
4. METAL DECK SHALL BE ATTACHED TO ALL SUPPORTS WITH 5/8" DIA. PUDDLE WELDS AT 12" O.C. AND 6" O.C. AT ALL PERIMETER SUPPORTS. PROVIDE A MINIMUM OF TEN #10 TEK SCREWED SIDELAP CONNECTION PER TRUSS BAYS OR AS SHOWN ON PLANS.

SAWN LUMBER
1. ALL GRADES OF LUMBER SHALL BE RATED BY THE SOUTHERN PINE INSPECTION BUREAU (SPIB), OR THE WESTERN WOOD PRODUCTS ASSOCIATION (WWPA). LUMBER GRADES SHALL BE AS FOLLOWS, WITH A MAXIMUM MOISTURE CONTENT OF 19%:
   A. SOUTHERN PINE NO. 1.
   B. DOUGLAS FIR-LARCH NO. 1.
   C. HEM-FIR NORTH NO. 1
2. BOLT HEADS AND NUTS BEARING ON WOOD SHALL BE PROVIDED WITH STANDARD CUT WASHERS. ALL WOOD IN CONTACT WITH CONCRETE OR MASONRY SHALL BE PRESSURE TREATED.
3. MINIMUM NAILED CONNECTIONS FOR WOOD FRAMING MEMBERS SHALL BE IN ACCORDANCE WITH THE FLORIDA BUILDING CODE 2014.
4. CONNECTORS SHOWN ON THE DETAILS ARE MANUFACTURED BY SIMPSON. WRITTEN APPROVAL BY ENGINEER REQUIRED FOR SUBSTITUTIONS.

WALL SHEATHING
1. ALL SHEATHING SHALL CONFORM TO AMERICAN PLYWOOD ASSOCIATION (APA) DESIGN SPECIFICATION, LATEST EDITION.
2. SHEATHING SHALL BE CONTINUOUS OVER THREE ADJACENT SPANS MINIMUM. WALL SHEATHING SHALL BE 5/8" & 3/4" EXTERIOR GRADE WOOD SHEATHING. FASTEN TO SUPPORTS WITH #10 TEK SCREWS @ 6" O.C. AT ALL PANEL EDGES AND 12" ON CENTER AT INTERMEDIATE SUPPORTS, U.N.O.
3. REFER TO DRAWINGS FOR SPECIAL SHEATHING OR NAILING REQUIREMENTS. PROVIDE SIMPSON "PSCL" PANEL CLIPS, MIN. 1 CLIP PER SIDE TO ALLOW FOR EXPANSION. THE SHEATHING SHALL NOT BE USED AS A NAILING EDGE.

MASONRY
CONCRETE BLOCK DESIGN AND CONSTRUCTION SHALL CONFORM TO"BUILDING CODE REQUIREMENTS FOR MASONRY STRUCTURES," TMS 402/ACI 530/ASCE 5 AND "SPECIFICATIONS FOR MASONRY STRUCTURES" (TMS 602/ACI 530.1/ASCE 6)
1. MASONRY MATERIALS SHALL CONFORM TO THE LATEST EDITION OF THE FOLLOWING SPECIFICATIONS:
   A. HOLLOW LOAD BEARING CONCRETE BLOCK: ASTM C-90, GRADE N1. MINIMUM COMPRESSIVE STRENGTH = 1900 PSI AT 28 DAYS.
   B. MORTAR: ASTM C-270, TYPE S. MINIMUM COMPRESSIVE STRENGTH = 1800 PSI AT 28 DAYS.
   C. MORTAR: ASTM C-270, TYPE M. MINIMUM COMPRESSIVE STRENGTH = 2500 PSI AT 28 DAYS. (USED FOR BELOW GRADE WORK)
   D. GROUT: ASTM C-476. MINIMUM COMPRESSIVE STRENGTH = 2000 PSI AT 28 DAYS
   E. MASONRY REINFORCEMENT: ASTM A-82 GALVANIZED
   F. MASONRY PRISM STRENGTH: F'm = 1500 PSI
2. PRIOR TO DELIVERY OF MASONRY UNITS TO THE JOB SITE, FURNISH TO THE OWNER AFFIDAVITS FROM AN APPROVED TESTING LABORATORY CERTIFYING THAT ALL UNITS CONFORM TO THEIR RESPECTIVE ASTM REQUIREMENTS.
3. GROUT ALL CAVITIES CONTAINING REINFORCEMENT IN LIFTS NOT TO EXCEED 5'-0".
4. LABORATORY PREPARED MIXES SHALL BE PREPARED AND TESTED IN ACCORDANCE WITH ASTM C-270. FIELD MORTAR SHALL BE TESTED BY AN APPROVED TESTING LABORATORY IN ACCORDANCE WITH ASTM C-780 TWO SETS OF THREE MORTAR CUBES SHALL BE TAKEN DIRECTLY FROM THE MIXER FOR EACH DAY OF MASONRY WORK. TEST THE CUBES AT 28 DAYS. ACCEPTANCE OF THE MORTAR SHALL BE AT THE DISCRETION OF THE ENGINEER.
5. CALCIUM CHLORIDE AND/OR ADMIXTURES CONTAINING CALCIUM CHLORIDE SHALL NOT BE INCLUDED IN MORTAR OR GROUT MIX, EXCEPT WHEN APPROVED IN WRITING BY THE STRUCTURAL ENGINEER. NO ANTI FREEZE COMPOUNDS SHALL BE USED TO LOWER THE MORTAR'S FREEZING POINT.
6. NO EXTERIOR MASONRY SHALL BE LAID WHEN THE OUTSIDE AIR TEMPERATURE IS LESS THAN 40 DEGREES FAHRENHEIT, UNLESS THE RECOMMENDATIONS SPECIFIED BY THE INTERNATIONAL MASONRY INDUSTRY ALL WEATHER COUNCIL IN THEIR PUBLICATION "RECOMMENDED PRACTICES AND GUIDE SPECIFICATIONS FOR COLD WEATHER MASONRY" ARE STRICTLY FOLLOWED.
7. THE MASONRY CONTRACTOR SHALL PROVIDE BRACING TO WITHSTAND HORIZONTAL PRESSURES AS REQUIRED BY THE BUILDING CODE AND LOCAL ORDINANCE.

SHOP DRAWINGS
1. SHOP DRAWING SUBMITTALS SHALL CONSIST OF A MINIMUM OF 2 REPRODUCIBLES OF EACH DRAWING.
2. SHOP DRAWINGS SHALL BE REVIEWED BY CONTRACTOR TO VERIFY THAT SUBMITTAL IS COMPLETE PRIOR TO SUBMITTING TO ARCHITECT/ENGINEER.
3. SHOP DRAWING SUBMITTALS SHALL INCLUDE THE FOLLOWING:
   A. CONCRETE MIX DESIGN
   B. FOUNDATION REINFORCING BARS
   C. STRUCTURAL STEEL
   D. STRUCTURAL MASONRY
   E. STEEL JOISTS AND CALCULATIONS
   F. METAL DECK
   G. TRELLIS SYSTEM & CALCULATIONS

SPECIAL INSPECTIONS
1. SPECIAL INSPECTIONS SHALL BE PERFORMED IN ACCORDANCE WITH SECTION 110 OF FBC 2014 AND THE OWNER SHALL EMPLOY ONE OR MORE SPECIAL INSPECTORS TO PROVIDE INSPECTIONS DURING CONSTRUCTION ON THE TYPES OF WORK LISTED UNDER SECTION 1704. THE FOLLOWING AREAS OF WORK REQUIRE SPECIAL INSPECTIONS IN ACCORDANCE WITH THE LISTED 2012 IBC SECTIONS/LOCATIONS:
   A. SOILS - SECTION 1704.7 PER TABLE 1704.7.
   B. CONCRETE - SECTION 1704.4 PER TABLE 1704.4.
   C. STEEL - SECTION 1704.3 PER TABLE 1704.3.
   D. MASONRY - SECTION 1704.5 PER TABLE 1704.5.1.

MISCELLANEOUS
1. ALL DIMENSIONS ON STRUCTURAL DRAWINGS TO BE CHECKED AGAINST ARCHITECTURAL, MECHANICAL AND ELECTRICAL DRAWINGS BY THE GENERAL CONTRACTOR AND ANY DISCREPANCIES ARE TO BE REPORTED TO THE ARCHITECT IMMEDIATELY.
2. THE CONTRACTOR SHALL ASSUME RESPONSIBILITY, UNRELIEVED BY REVIEW OF SHOP DRAWINGS OR PERIODIC OBSERVATION OF CONSTRUCTION, FOR COMPLIANCE WITH THE CONTRACT DOCUMENTS, FOR FABRICATION PROCESSES AND CONSTRUCTION TECHNIQUES, AND FOR SAFE CONDITIONS ON THE JOB SITE.
3. DO NOT SCALE THE DRAWINGS.

CONCRETE BLOCK JOINT REINFORCEMENT:
ALL CONCRETE BLOCK WALLS TO RECEIVE THE FOLLOWING JOINT REINFORCEMENT:
LADDER OR TRUSS TYPE JOINT REINFORCING WITH SIDE AND CROSS RODS WITH WIRE SIZE (W2.8 OR 3/16" DIA.) SPACED 16" O.C. VERTICALLY. (HOHMANN & BARNARD 220 "SUPER HEAVY DUTY" OR EQUAL) SIMILAR FOR CONCRETE BRICK PRODUCTS.
ALL MULTI-WYTHE WALLS USE LADDER REINFORCING @ 16" O.C. TO TIE WYTHES TOGETHER WITH WIRE SIZE (W2.8 OR 3/16" DIA. - USE HOHMANN & BERNARD 240 LADDER MESH TWIN).

REQUIRED SPECIAL INSPECTIONS

(BY TESTING AGENCY)
In addition to the regular inspections required by Section 110 of the FBC 2014, the following items require special inspection in accordance with Section 1704 & 1707 of the IBC 2012.

ITEM	SECTION
INSPECTION OF FABRICATORS	
• VERIFY FABRICATION/QUALITY CONTROL PROCEDURES	1704.2
STEEL CONSTRUCTION	
• FIELD WELDING	1704.3.1
• DETAILS	1704.3.2
• HIGH-STRENGTH BOLTS	1704.3.3
CONCRETE CONSTRUCTION	
• REINFORCING STEEL INSTALLATION	TABLE 1704.4
• CAST-IN PLACE ANCHOR BOLTS	TABLE 1704.4
• VERIFY DESIGN MIX	TABLE 1704.4
• FRESH CONCRETE SAMPLING	TABLE 1704.4
• CONCRETE PLACEMENT	TABLE 1704.4
• CONCRETE CURING OPERATIONS	TABLE 1704.4
• EVALUATION OF CONCRETE STRENGTH	TABLE 1704.4
MASONRY CONSTRUCTION	
• VERIFY PROPORTIONS OF SITE PREPARED MORTAR AND GROUT	TABLE 1704.5.1
• VERIFY CONSTRUCTION OF MORTAR JOINTS.	TABLE 1704.5.1
• VERIFY LOCATION OF REINFORCEMENT AND CONNECTORS.	TABLE 1704.5.1
• VERIFY SIZE AND LOCATION OF STRUCTURAL MASONRY ELEMENTS	TABLE 1704.5.1
• VERIFY TYPE, SIZE, AND LOCATION OF ANCHORS, INCLUDING DETAILS OF ANCHORAGE OF MASONRY TO STRUCTURAL MEMBERS, FRAMES OR OTHER CONSTRUCTION.	TABLE 1704.5.1
• VERIFY SIZE, GRADE, AND TYPE OF REINFORCEMENT.	TABLE 1704.5.1
• VERIFY PROTECTION OF MASONRY DURING HOT/COLD WEATHER	TABLE 1704.5.1
• VERIFY GROUT SPACE IS CLEAN PRIOR TO GROUTING.	TABLE 1704.5.1
• VERIFY GROUT PLACEMENT COMPLIES WITH CODE AND CONSTRUCTION DOCUMENT PROVISIONS.	TABLE 1704.5.1
• OBSERVE PREPARATION OF GROUT SPECIMENS, MORTAR SPECIMENS, AND/OR PRISMS.	TABLE 1704.5.1
SOILS	
• VERIFY MATERIALS BELOW ARE ADEQUATE TO ACHIEVE DESIGN BEARING CAPACITY.	TABLE 1704.7
• VERIFY EXCAVATIONS ARE EXTENDED TO PROPER DEPTH AND HAVE REACHED PROPER MATERIAL.	TABLE 1704.7
• PERFORM CLASSIFICATION AND TESTING OF CONTROLLED FILL MATERIALS.	TABLE 1704.7
• VERIFY SITE PREPARATION WITH SOILS REPORT	TABLE 1704.7
• VERIFY USE OF PROPER MATERIALS, DENSITIES AND LIFT THICKNESS DURING PLACEMENT AND COMPACTION OF CONTROLLED FILL.	TABLE 1704.7
COLD-FORMED STEEL FRAMING	
• PERIODIC INSPECTION DURING WELDING OPERATIONS OF ELEMENTS OF THE SEISMIC-FORCE-RESISTING SYSTEM.	1707.4
• PERIODIC INSPECTIONS FOR SCREW ATTACHMENT, BOLTING, ANCHORING AND OTHER FASTENING OF COMPONENTS WITHIN THE SEISMIC-FORCE-RESISTING SYSTEM.	1707.4

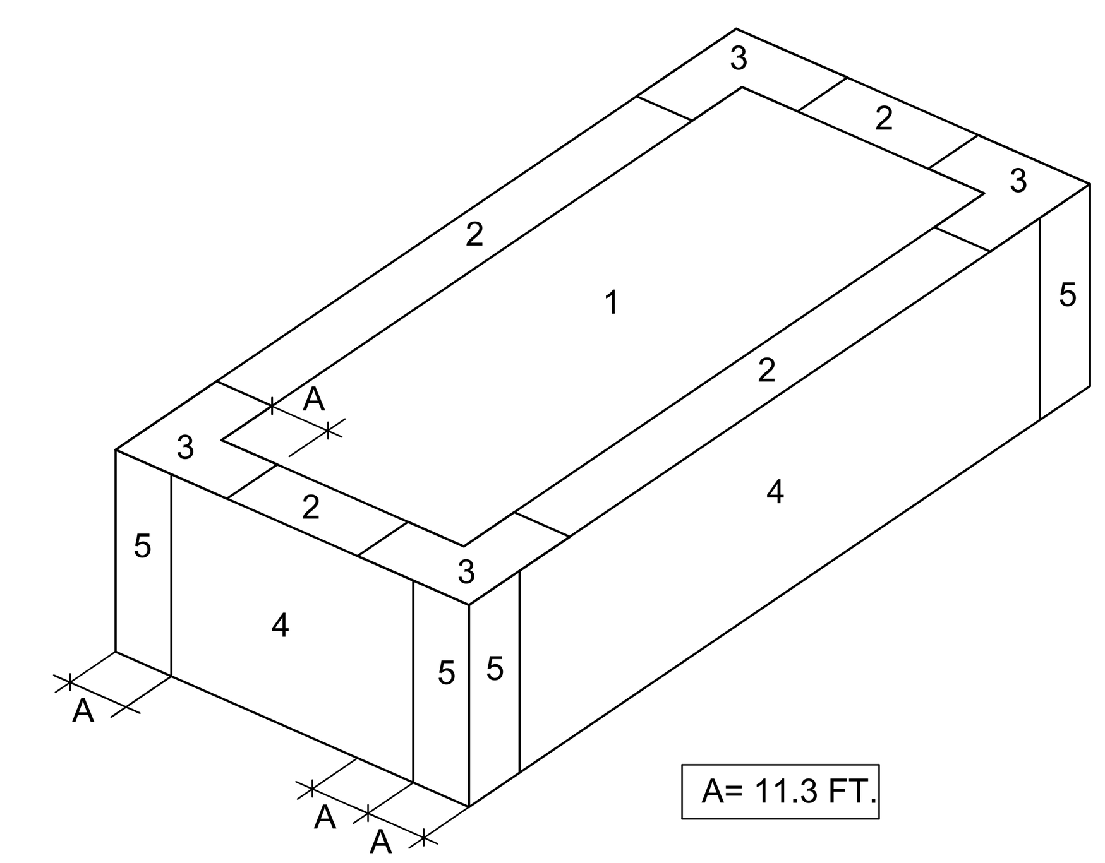
REQUIRED SPECIAL INSPECTIONS NOTES:
1. REFER TO PROJECT SPECIFICATION FOR ADDITIONAL QUALITY CONTROL/QUALITY ASSURANCE REQUIREMENTS.
2. GENERAL CONTRACTOR SHALL COORDINATE ANY ADDITIONAL SPECIAL INSPECTION REQUIREMENTS WITH OWNER AND APPLICABLE BUILDING AUTHORITIES.

ASCE 7-10 / ULTIMATE COMPONENT AND CLADDING ROOF PRESSURE

ZONE	EFFECTIVE WIND AREA	COEFFICIENT	ULTIMATE DWP PNET (psf)		NOMINAL DWP PNET (psf)	
1	10	1.14	34.41	-84.59	20.65	-50.76
1	20	1.14	30.83	-84.59	18.50	-50.76
1	50	1.14	28.68	-81.01	17.21	-48.61
1	100	1.14	27.24	-77.43	16.35	-46.46
2	10	1.14	84.59	-141.95	50.76	-85.17
2	20	1.14	81.01	-127.61	48.61	-76.57
2	50	1.14	75.28	-106.10	45.17	-63.66
2	100	1.14	71.69	-91.76	43.01	-55.06
3	10	1.14	84.59	-141.95	50.76	-85.17
3	20	1.14	81.01	-127.61	48.61	-76.57
3	50	1.14	75.28	-106.10	45.17	-63.66
3	100	1.14	71.69	-91.76	43.01	-55.06

ASCE 7-10 / ULTIMATE COMPONENT AND CLADDING WALL PRESSURE

ZONE	EFFECTIVE WIND AREA	COEFFICIENT	ULTIMATE DWP PNET (psf)		NOMINAL DWP PNET (psf)	
4	10	1.14	84.59	-91.76	50.76	-55.06
4	20	1.14	81.01	-88.18	48.61	-52.91
4	50	1.14	75.28	-83.16	45.17	-49.90
4	100	1.14	71.69	-78.86	43.01	-47.32
4	200	1.14	68.82	-75.99	41.29	-45.60
5	10	1.14	84.59	-113.27	50.76	-67.96
5	20	1.14	81.01	-106.10	48.61	-63.66
5	50	1.14	75.28	-95.35	45.17	-57.21
5	100	1.14	71.69	-88.18	43.01	-52.91
5	200	1.14	68.82	-81.01	41.29	-48.61



Jose M Ortiz Ruiz

Digitally signed by Jose M Ortiz Ruiz
DN: c=US, o=IdenTrust ACES Business Representative, ou=CPH INC, cn=Jose M Ortiz Ruiz, 0.9.2342.19200300.100.1.1=A01097C00000151D0943AF20000A3D5
Date: 2017.02.15 10:59:48 -05'00'

OPPIDAN
builder of towns. creator of value.
Orchard
SUPPLY HARDWARE

NAPLES NORTH

JOSE MIGUEL ORTIZ
LICENSE No. 67920
STATE OF FLORIDA
PROFESSIONAL ENGINEER

2/13/2017

cph
www.cphcorp.com
A Full Service A & E Firm
Eng. C.O.A. No. 3215 - 500 West Fulton Street Sanford, Florida 32771

ADDENDUM 02

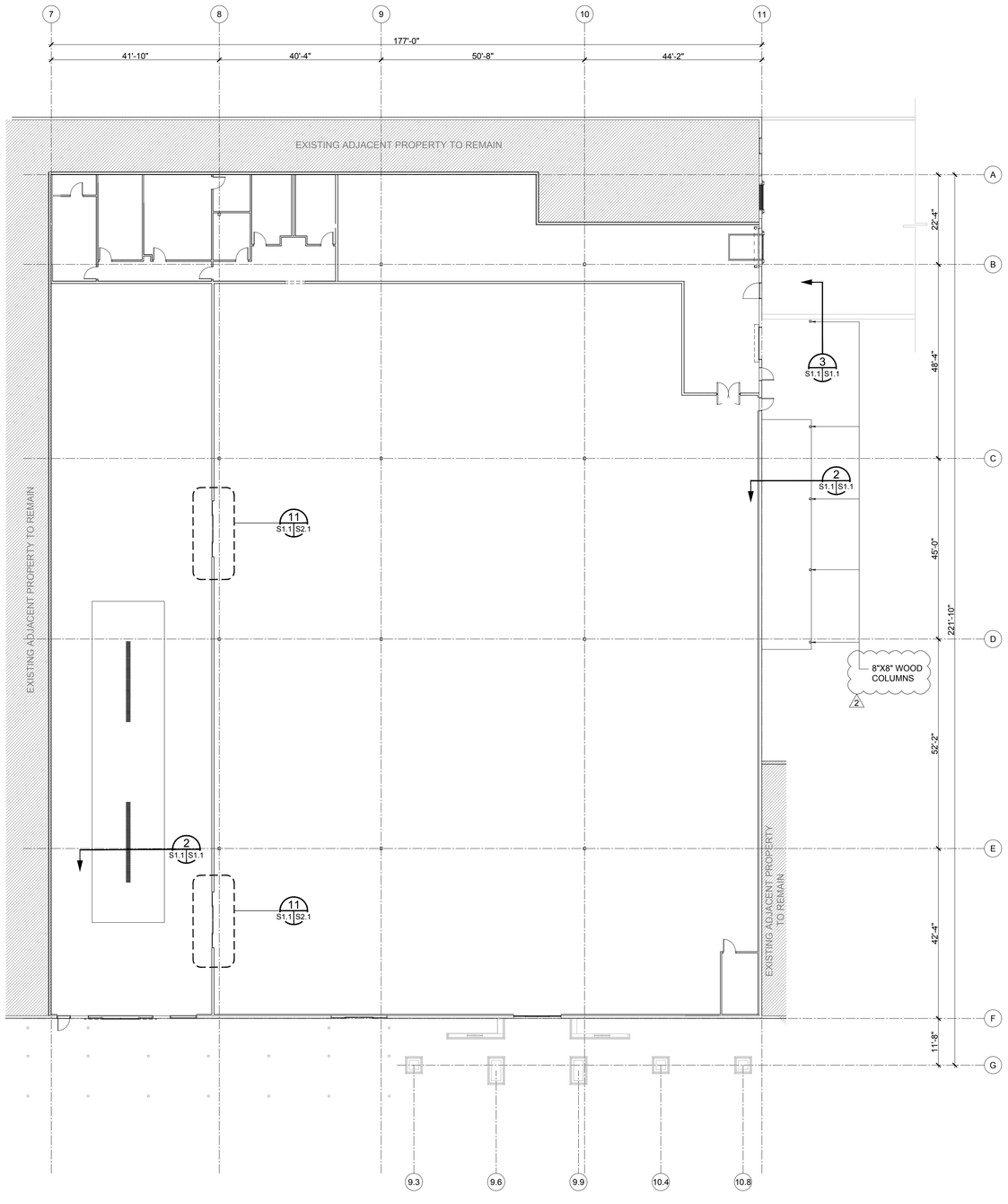
STRUCTURAL GENERAL NOTES

1000 IMMOKALEE ROAD, NAPLES, FL 34110

S1.0
36-16117-00
11-21-2016
REVISIONS

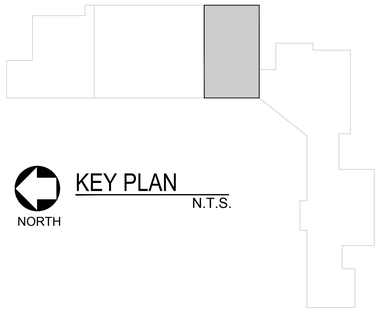
DLR Group
Architecture Engineering Planning Interiors

10/18/2016 8:43:16 AM



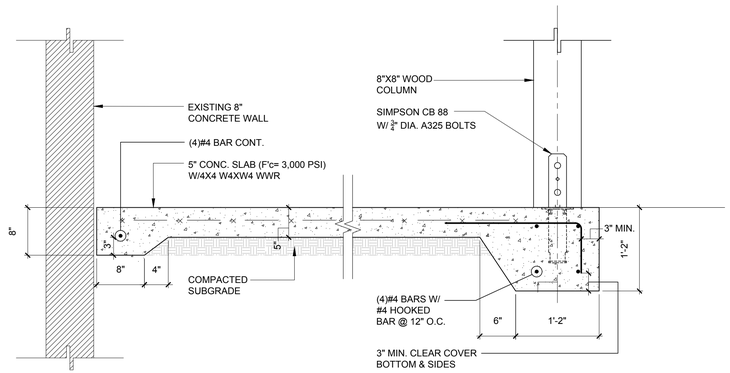
1 S1.1 CONCRETE REPAIR PLAN
1/16" = 1'-0"

NORTH

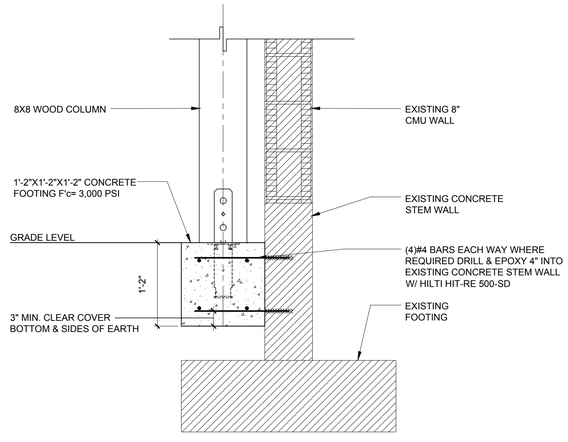


KEY PLAN
N.T.S.

NORTH



2 S1.1 S1.1 OUTDOOR SLAB DETAIL
1" = 1'-0"



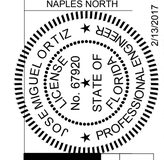
3 S1.1 S1.1 COLUMN FOOTING
1" = 1'-0"

FOUNDATION NOTES

1. TOP OF EXISTING SLAB = 0'-0". 0'-0" IS FOR REFERENCE ONLY. SEE CIVIL FOR NVGD ELEVATION.
2. SEE ARCH. FOR DIMENSIONS NOT SHOWN.
3. SEE ARCHITECTURAL AND MECHANICAL DRAWINGS FOR ALL SLEEVES, PIPES, INSERTS AND EMBEDDED ITEMS.
4. SEE ARCHITECTURAL AND MECHANICAL DRAWINGS FOR SLOPES & RECESSES.
5. WHERE REQUIRED, NEW SLAB SHALL BE 5" CONC. SLAB ON GRADE (P'c=3000 psi) W/ 6X6 W2.1XW2.1 WWF ON 10 MIL VAPOR BARRIER ON PREPARED SUBGRADE PER GEOTECHNICAL REPORT.
6. F-# DENOTES NEW CONCRETE FOOTING, SEE SCHEDULE ON THIS SHEET.
7. T.E. DENOTES NEW CONCRETE THICKENED EDGE, SEE SECTION.
8. TOP OF FOOTING ELEVATION = 1'-6" (UNO).
9. WHERE REQUIRED DRILL & EPOXY WITH HILTI HIT-RE 500-SD.

OPPIDAN
builder of towns, creator of value.
Orchard SUPPLY HARDWARE

NAPLES NORTH



2/13/2017

cph
www.cphcorp.com
A Full Service A & E Firm
Engs. C.O.A. No. 3215 - 500 West Fulton Street Sanford, Florida 32771

ADDENDUM 02

FOUNDATION REPAIR AND DETAILS

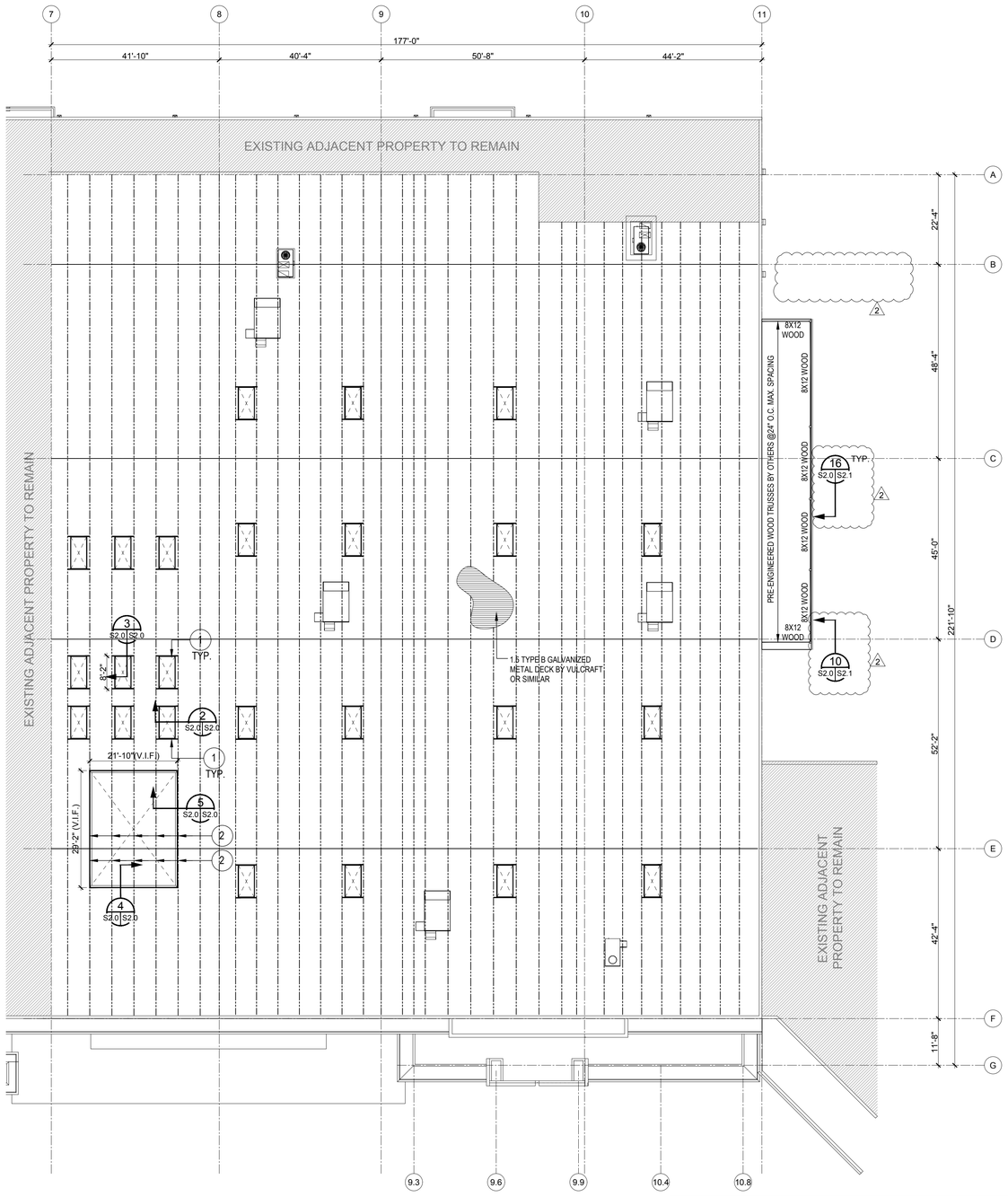
1000 IMMOKALEE ROAD, NAPLES, FL 34110

S1.1

36-16117-00
1" = 2'-0"
10/18/2016
REVISIONS
2 2/08/2017 ADDENDUM 02

DLR Group
Architecture Engineering Planning Interiors

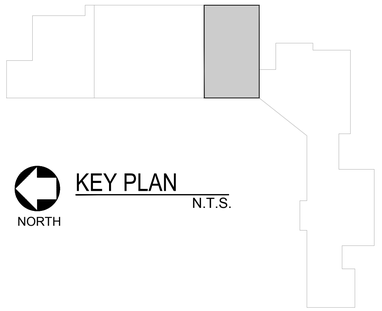
10/18/2016 8:43:16 AM



1 S2.0 ROOF FRAMING PLAN
1/16" = 1'-0"

NORTH

ROOF DECK:
A) DECK AND ANCHORAGE SHALL BE PER STEEL DECK INSTITUTE (SDI) "SPECIFICATIONS FOR STEEL ROOF DECK".
B) METAL ROOF DECK SHALL BE GALVANIZED AND SIZE AND GAUGE SHOWN ON DRAWINGS.
C) DECK MANUFACTURER SHALL PROVIDE RIDGE AND VALLEY PLATES, STEEL CANT. STRIPS AND SUMP PANS AS REQUIRED TO PROVIDE A FINISH SURFACE FOR INSULATION AND ROOFING.
D) DECK LAY-UP SHALL BE MULTIPLE SPANS, WELDED TO SUPPORT.
E) METAL DECK SHALL BE WIDE RIB WITH NESTING SIDE SEAMS OF DEPTH AND GAGE INDICATED ON THE DRAWINGS.
F) ATTACHMENT: METAL DECK SHALL BE ATTACHED TO ALL SUPPORTING STEEL WITH PUDDLE WELDS (5/8"Ø MINIMUM) OR POWER ACTUATED FASTENERS AT 12" CENTERS (5 PER SHEET MINIMUM). SIDE LAP CONNECTIONS SHALL BE MADE WITH #10 SELF TAPPING SCREWS AT A MAXIMUM OF 3'-0" CENTERS.



KEY PLAN
N.T.S.

NORTH

NOTES

1- PRE-MANUFACTURED WOOD TRUSS CANOPIES ARE TO BE PROVIDED BY OTHERS. REFER TO ARCH DRAWINGS FOR CANOPY LOCATION.

2- REFER TO FIRE PROTECTION PLANS FOR REQUIRED SUPPORT & OPENINGS RELATED TO THE FIRE PROTECTION SYSTEM.

ROOF FRAMING LEGEND

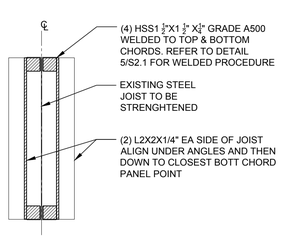
EXISTING STEEL JOIST

NEW SKYLIGHT STRUCTURAL FRAMING

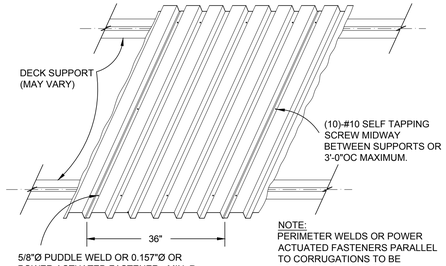
1 STRUCTURAL FRAMING ANGLE L5x5x3/8. TYP.

2 PROVIDE STEEL JOIST STRENGHTENING ELEMENTS. REFER TO DETAILS 4/S2.0, 5/S2.1 & 6/S2.1

3 REFER TO S2.1 FOR RTU OPENING FRAMING



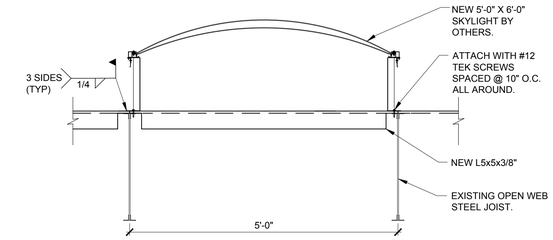
4A S2.0 S2.0 STEEL JOIST STRENGHTENING
1" = 1'-0"



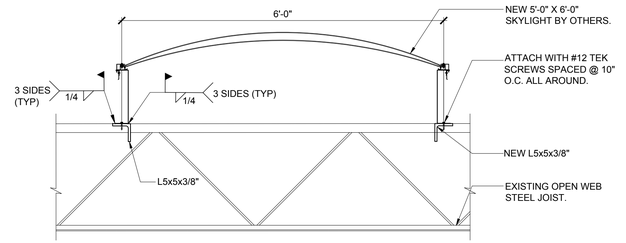
TYPICAL DECK ATTACHMENT DETAIL

DECK ATTACHMENT NOTES:
TIGHT DECK TO SUPPORT CONTACT SHALL BE MAINTAINED AT ALL WELD LOCATIONS.

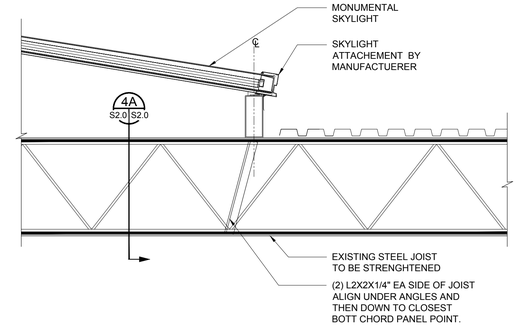
WELDING ROD AMPERAGE AND BURN OFF RATE SHALL BE DETERMINED BY FIELD TESTING AS FOLLOWS:
A 20'-0" LONG SECTION OF DECK SHALL BE ANCHORED TO A STEEL BEAM WITH TWO WELDS AT 6"OC. LEVERAGE SHOULD BE APPLIED TO THE PANEL. YIELDING SHOULD NOT BE NOTED AROUND WELD AND SUDDEN SEPARATION SHOULD NOT OCCUR ON THE CONTACT PLANE. SHOULD EITHER OCCUR, WELDING TIME AND/OR WELDING AMPERAGE SHALL BE ADJUSTED UNTIL ADEQUATE WELDING IS ACCOMPLISHED.



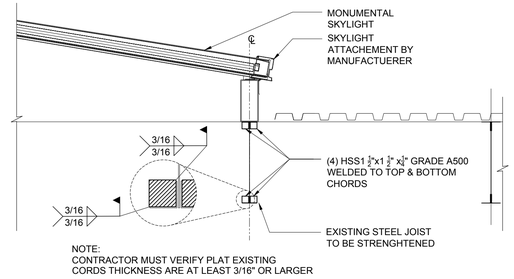
2 S2.0 S2.0 ALUMINUM SKYLIGHT TO JOIST SECTION
3/4" = 1'-0"



3 S2.0 S2.0 ALUMINUM SKYLIGHT TO JOIST SECTION
3/4" = 1'-0"



4 S2.0 S2.0 SKYLIGHT FRAMING DETAIL
3/4" = 1'-0"



5 S2.0 S2.0 SKYLIGHT FRAMING DETAIL
3/4" = 1'-0"

OPPIDAN
builder of towns, creator of value.
Orchard SUPPLY HARDWARE

NAPLES NORTH
JOSE MIGUEL ORTIZ LICENSE No. 67920 STATE OF FLORIDA PROFESSIONAL ENGINEER
2/13/2017

cph
www.cphcorp.com
A Full Service A & E Firm
Engs. C.O.A. No. 3215 - 500 West Fulton Street Sanford, Florida 32771

ADDENDUM 02

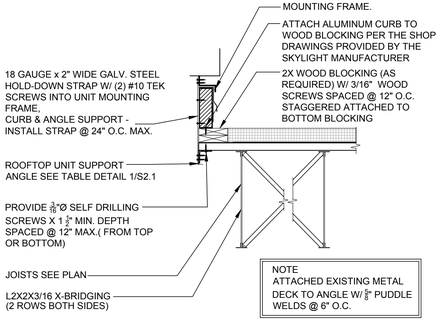
ROOF FRAMING PLAN

1000 IMMOKALEE ROAD, NAPLES, FL 34110

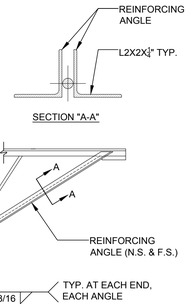
S2.0
16-16117-00
11-22-2016
REVISIONS
2 2/08/2017 ADDENDUM 02

DLR Group
Architecture Engineering Planning Interiors

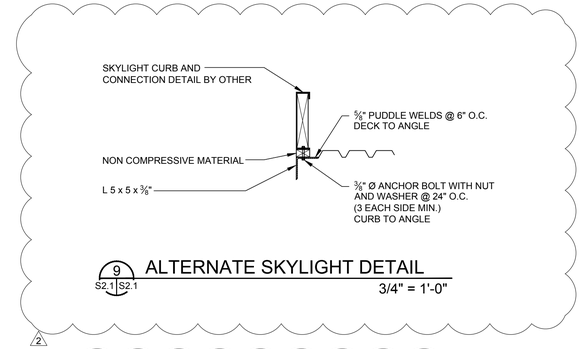
10/18/2016 8:43:16 AM



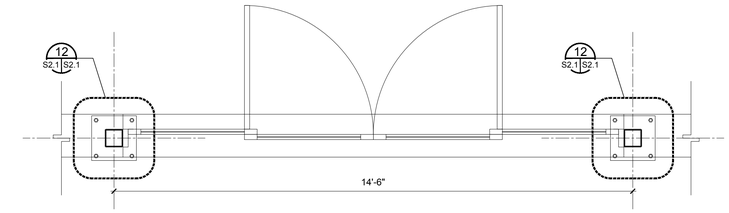
9 S2.1 S2.1 SECTION @ ROOF CURB 3/4" = 1'-0"



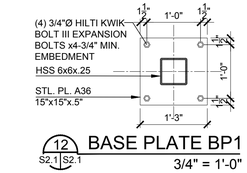
5 S2.1 S2.1 END DIAGONAL REINFORCING N.T.S.



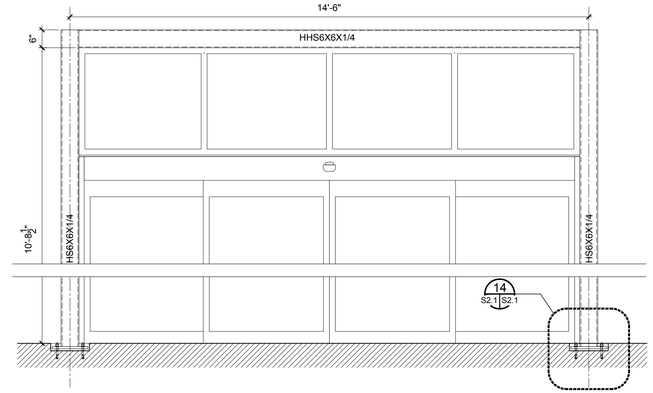
9 S2.1 S2.1 ALTERNATE SKYLIGHT DETAIL 3/4" = 1'-0"



11 S2.1 S2.1 ENTRY DOOR STRUCTURAL FRAMING (ACCESS FROM STORE TO NURSERY) 1/2" = 1'-0"



12 S2.1 S2.1 BASE PLATE BP1 3/4" = 1'-0"



13 S2.1 S2.1 ENTRY DOOR FRAMING ELEVATION (ACCESS FROM STORE TO NURSERY) 1/2" = 1'-0"

THE GENERAL CONTRACTOR SHALL VERIFY ALL RTU WEIGHTS, LOCATIONS, CURB DIMENSIONS, SUPPORT RACK DIMENSIONS, ROOF OPENING SIZES AND ROOF OPENING LOCATIONS WITH MECH. PRIOR TO FABRICATION AND CONSTRUCTION.

FOR SIZE AND LOCATION OF REQUIRED ROOF OPNG. SEE MECHANICAL DWGS. AND OR HVAC MANUFACTURER

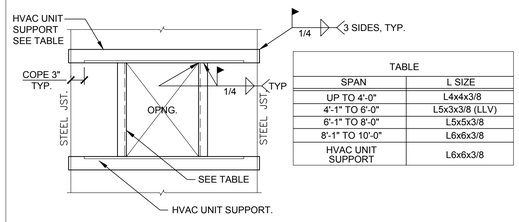
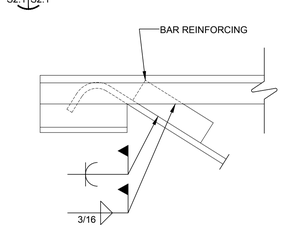
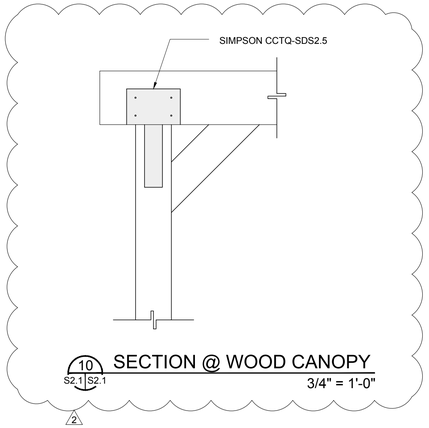


TABLE	
SPAN	L SIZE
UP TO 4'-0"	L4x4x3/8
4'-1" TO 6'-0"	L5x3x3/8 (LLV)
6'-1" TO 8'-0"	L5x5x3/8
8'-1" TO 10'-0"	L6x6x3/8
HVAC UNIT SUPPORT	L6x6x3/8

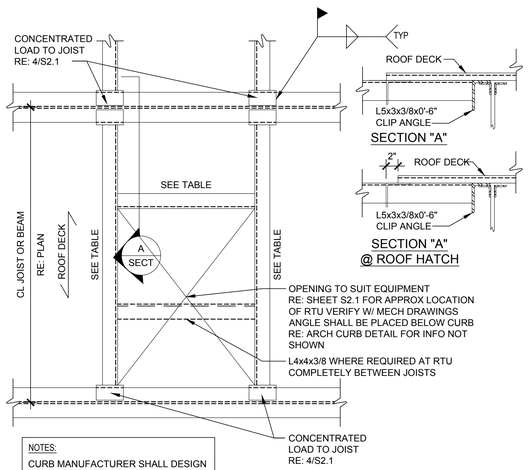
2 S2.1 S2.1 DETAIL @ ROOF OPENING 3/4" = 1'-0"



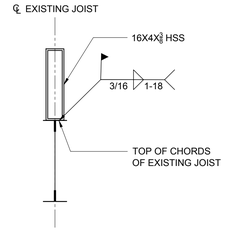
6 S2.1 S2.1 BAR ADDED FOR ADDITIONAL WELD ON END DIAGONAL N.T.S.



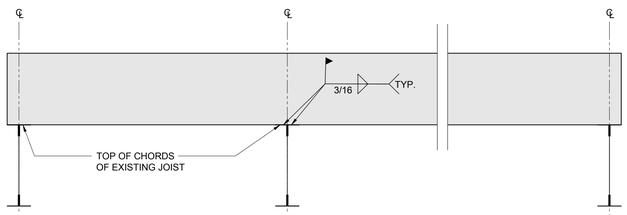
10 S2.1 S2.1 SECTION @ WOOD CANOPY 3/4" = 1'-0"



3 S2.1 S2.1 ROOF OPENING FRAME 1" = 1'-0"

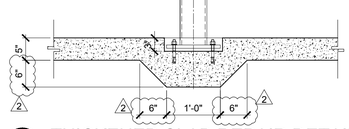


7 S2.1 S2.1 SECTION 3/4" = 1'-0"

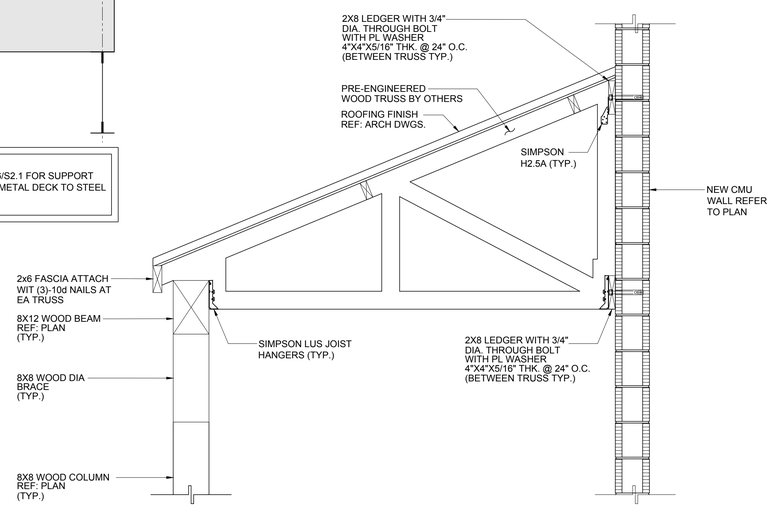


8 S2.1 S2.1 SECTION 3/4" = 1'-0"

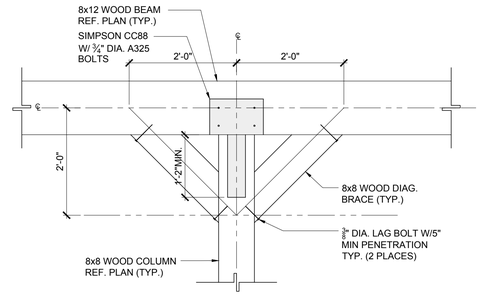
NOTE:
WHERE REQUIRED, PROVIDE ANGLES PER DETAIL 6/S2.1 FOR SUPPORT OF THE EXISTING METAL DECK. ATTACH EXISTING METAL DECK TO STEEL ANGLES WITH PUDDLE WELDS AT 6" ON CENTER.



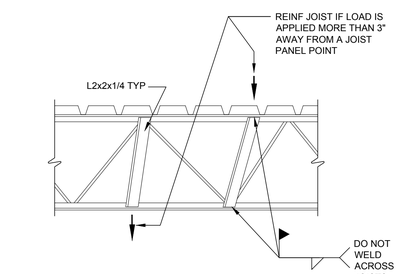
14 S2.1 S2.1 THICKENED SLAB REPAIR DETAIL 3/4" = 1'-0"



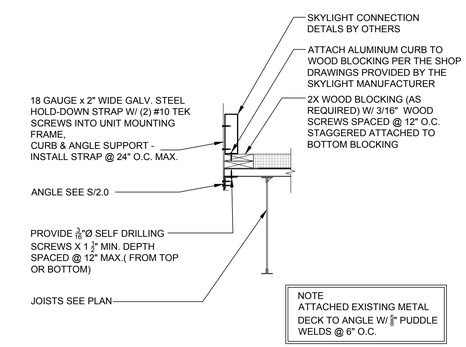
15 S2.1 SECTION AT WOOD CANOPY 3/4" = 1'-0"



16 S2.1 SECTION @ WOOD CANOPY 3/4" = 1'-0"



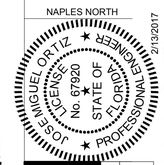
4 S2.1 S2.1 TYPICAL JOIST REINF AT CONCENTRATED LOADS 1" = 1'-0"



17 S2.1 S2.1 SKYLIGHT CONNECTION DETAIL 3/4" = 1'-0"

OPPIDAN builder of towns, creator of value.
Orchard SUPPLY HARDWARE

NAPLES NORTH



cph
www.cphcorp.com
A Full Service A & E Firm
Engs. C.O.A. No. 3215 - 500 West Fulton Street Sanford, Florida 32771

ADDENDUM 02

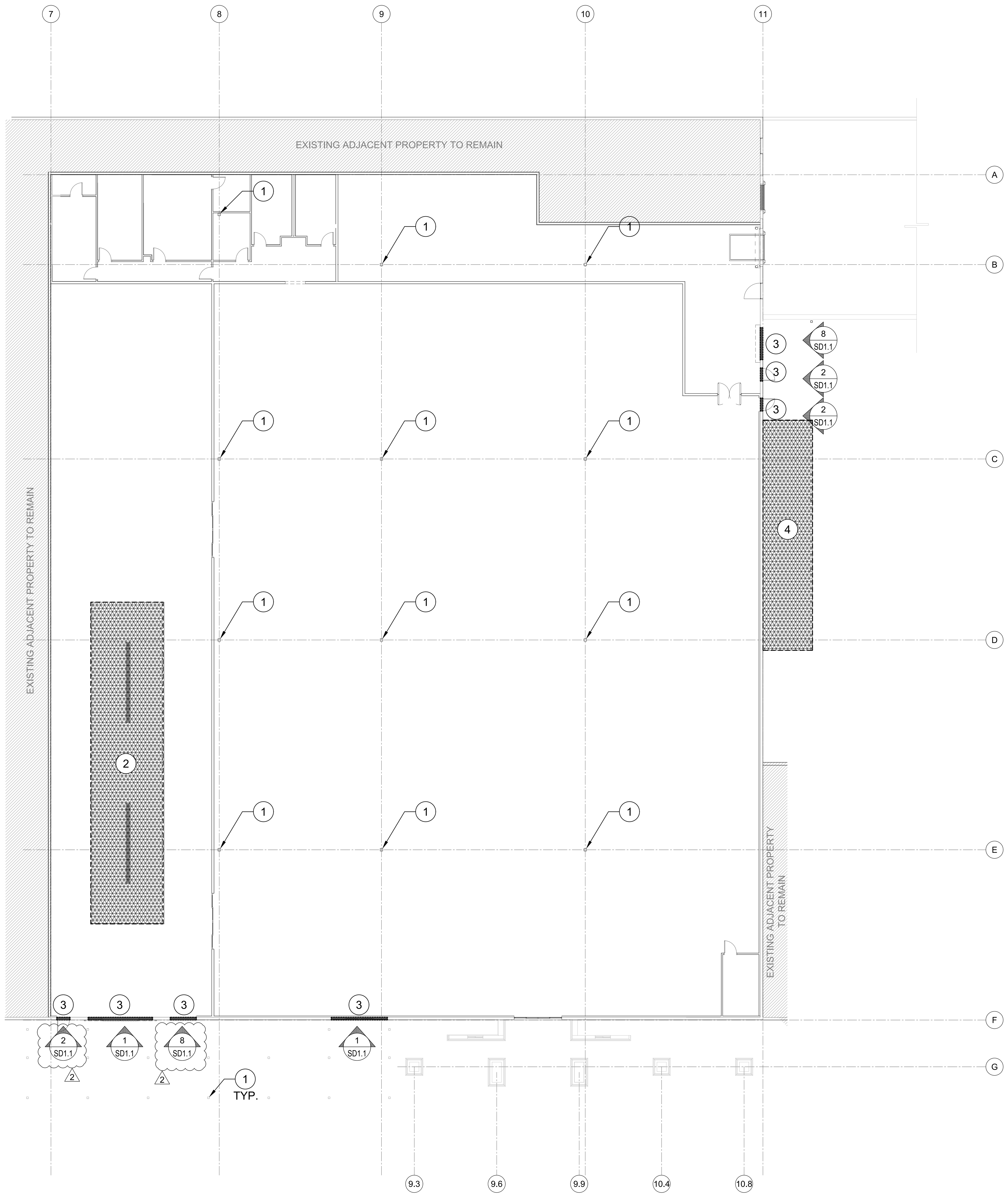
ROOF FRAMING SECTIONS AND DETAILS

1000 IMMOKALEE ROAD, NAPLES, FL. 34110

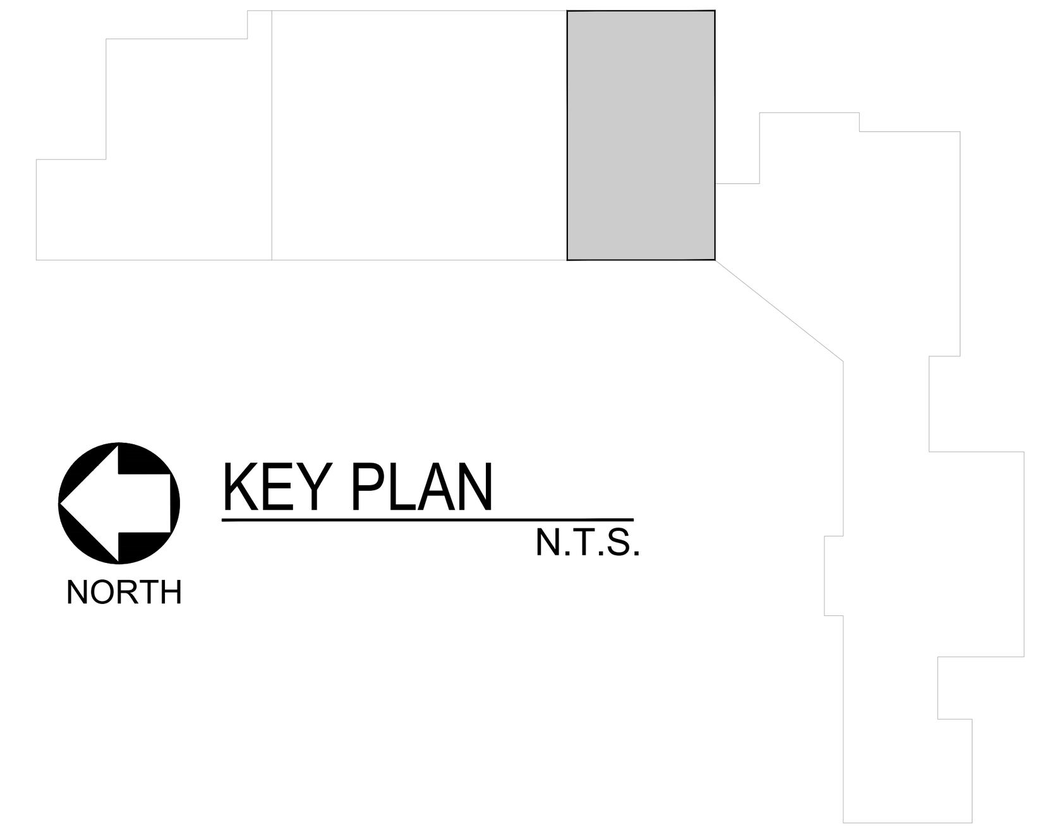
S2.1

36-16117-00
12-21-2016
REVISIONS
2 2/08/2017 ADDENDUM 02

DLR Group
Architecture Engineering Planning Interiors



DEMOLITION PLAN
1/16" = 1'-0"
NORTH



KEY PLAN
N.T.S.
NORTH

DEMOLITION NOTES

A. THE INTENT OF THE DEMOLITION PLANS IS TO SHOW THE GENERAL NATURE OF THE DEMOLITION SCOPE. THE GENERAL CONTRACTOR IS RESPONSIBLE FOR VISITING THE JOB SITE AND VERIFYING THE EXISTING CONDITION. THE GENERAL CONTRACTOR SHOULD NOTIFY THE ENGINEER IMMEDIATELY OF ANY DISCREPANCIES.

B. COORDINATE AND VERIFY WITH THE OWNER ALL ITEMS TO BE SALVAGED PRIOR TO DEMOLITION. THESE ITEMS MAY INCLUDE, BUT NOT LIMITED TO, THE FOLLOWING:

1) GENERATOR FUEL LINES
2) GENERATOR POWER SUPPLY CONDUITS AND WIRING.
3) GENERATOR SWITCH CONDUITS AND WIRING.

C. GENERAL CONTRACTOR IS TO MAINTAIN A SEPARATION BETWEEN AREAS WITHIN THE SCOPE OF WORK AND AREAS OUTSIDE OF THE SCOPE OF WORK BY PROVIDING PLASTIC SHEATHING BETWEEN CONTIGUOUS SPACES AND/OR TEMPORARILY TAPING OF JOINTS AND GAPS TO PREVENT DUST MIGRATION.

D. CAUSE NO DAMAGE TO EXISTING CONSTRUCTION TO REMAIN. TAKE CARE NOT TO ENCROACH ON ADJACENT OCCUPIED AREAS OR AREAS NOT WITHIN THE SCOPE OF WORK. PROTECT ALL EXISTING FINISHES, DOORS, FRAMES, ETC. WHICH ARE TO REMAIN.

E. CONDUCT DEMOLITION OPERATIONS & THE REMOVAL OF DEBRIS TO ENSURE MINIMUM INTERFERENCE WITH STREETS, WALKS, & OTHER ADJACENT OCCUPIED OR USED FACILITIES. COMPLY WITH LOCAL JURISDICTION REQUIREMENTS FOR RECYCLING AND TREATMENT OF ITEMS TO BE RECYCLED.

F. DISPOSE OF ALL DEMOLISHED OR REMOVED MATERIALS LEGALLY OFF THE SITE. COMPLY WITH ALL LOCAL HAULING & DISPOSAL REQUIREMENTS.

G. THE ENGINEER HAS NO KNOWLEDGE OF AND SHALL NOT BE HELD LIABLE FOR ANY ASBESTOS OR OTHER HAZARDOUS MATERIALS ON JOBSITE. THE CONTRACTOR SHALL IMMEDIATELY ISOLATE THE AFFECTED AREA IF ASBESTOS OR OTHER HAZARDOUS MATERIAL ARE DISCOVERED DURING CONSTRUCTION. NOTIFY OWNER FOR FURTHER INSTRUCTION BEFORE PROCEEDING WITH OTHER WORK.

H. MAINTAIN EXISTING UTILITIES TO REMAIN IN SERVICE AND PROTECT AGAINST DAMAGE DURING DEMOLITION OPERATIONS.

I. SCHEDULE ALL SERVICE SHUT-DOWN(S) WITH THE OWNER. NOTIFY OWNER A MINIMUM TIME OF ONE (1) WEEK AND ADDITIONALLY ONE (1) HOUR PRIOR TO SHUT-DOWN(S).

J. REMOVE ALL ABANDONED ANCHOR BOLTS & EMBEDDED ITEMS IN CONCRETE FLOORS THAT PROTRUDE ABOVE THE CONCRETE FLOOR SURFACE.

K. UPON COMPLETION, CLEAN THE ENTIRE AREA OF DEMOLITION TO A TIDY, UNIFORM CONDITION REMOVING ALL DEBRIS, DUST PARTITIONS & ASSOCIATED MATERIALS USED DURING THE DEMOLITION. CLEAN ALL AREAS IMPACTED BY THE DEMOLITION, INCLUDING BUT NOT LIMITED TO, ADJACENT OCCUPIED AREAS AND AREAS NOT WITHIN THE SCOPE OF WORK.

L. ABANDONING ITEMS OR UNUSED UTILITIES IN PLACE IS STRICTLY PROHIBITED, UNLESS SPECIFICALLY PERMITTED BY THE OWNER.

M. PATCH / PAINT / FINISHES:
TAPE, PATCH, SAND SMOOTH, AND PAINT ALL EXISTING INTERIOR WALLS WHERE DAMAGED TO UPGRADE TO CLIENT-ACCEPTED CONDITION. ALL ADJACENT EXISTING FINISHES DAMAGED OR AFFECTED BY DEMOLITION OR CONSTRUCTION OF NEW AREAS IN SCOPE OF WORK SHALL BE PATCHED AND REPAIRED TO MEET CLIENT SATISFACTION.

N. COMPLY WITH ALL STANDARD LOCAL, NATIONAL, STATE AND FEDERAL SAFETY REQUIREMENTS FOR DEMOLITION.

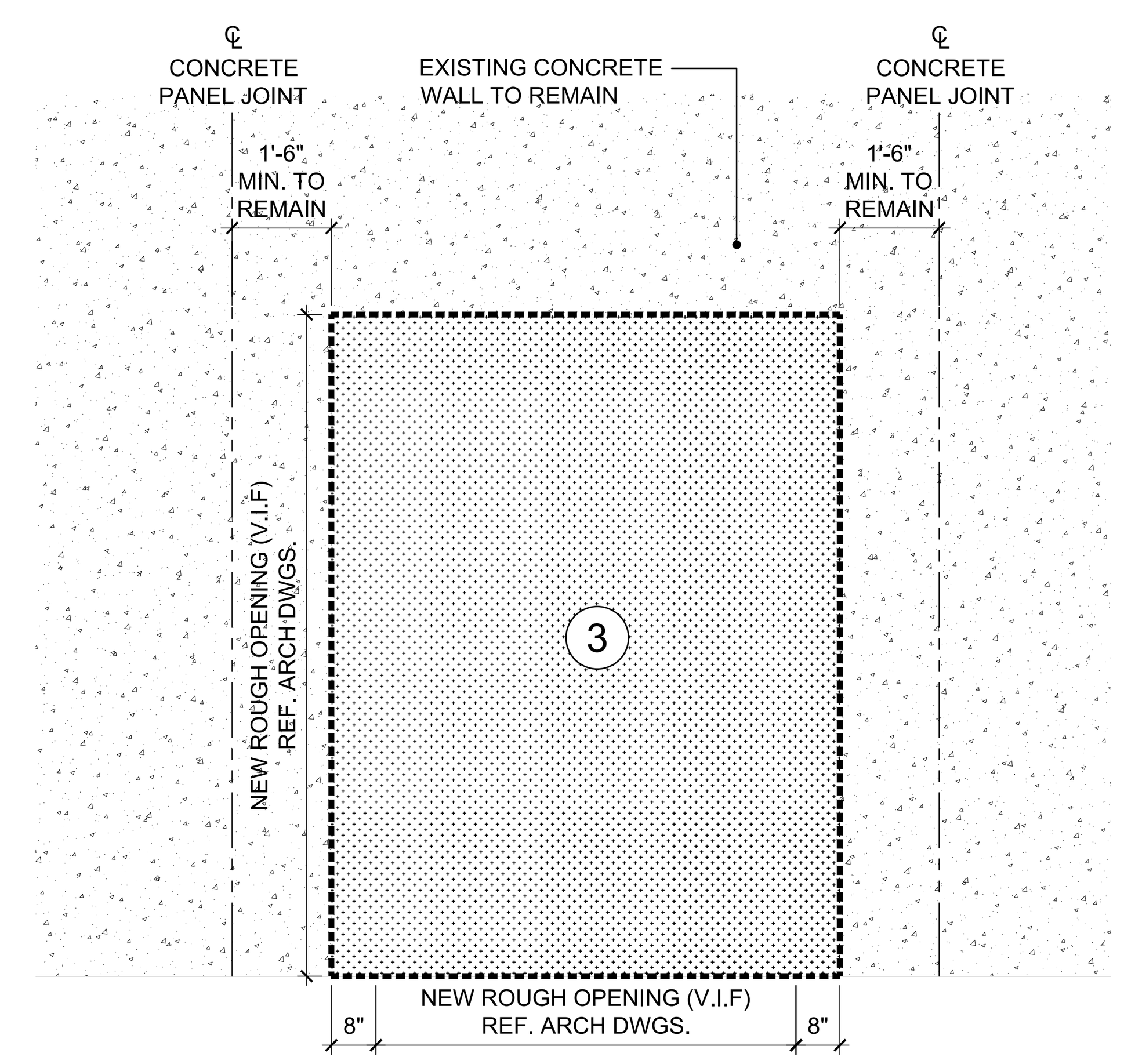
O. EXISTING STEEL SHALL BE TREATED FOR CORROSION WHERE VISIBLE SIGNS OF CORROSION EXIST.

P. THE CONTRACTOR SHALL TAKE CARE WHEN REMOVING EXISTING MASONRY WALLS AND CONCRETE AT LOCATIONS WHERE ROOF JOISTS, BRIDGING AND BRACING PASS THROUGH THE EXISTING MASONRY WALLS AND CONCRETE. THE USE OF SAW CUTTING AND HAND TOOLS MUST BE USED TO PREVENT DAMAGE TO EXISTING ROOF FRAMING.

Q. CONFIRM LIMITS OF DEMOLITION WITH ARCHITECT.

NOTE

REFER TO ARCHITECTURE AND PLUMBING PLANS FOR ADDITIONAL SLAB DEMO LOCATION



EXISTING WALL DEMOLITION
1/4" = 1'-0"

WALL OPENINGS DEMOLITION NOTES:

1- PROVIDE #5 REBAR A706 AS SHOWN AT EDGES OF OPENINGS W/ NON SHRINK, NON METALLIC MORTAR F'C=5000 PSI
2- PROVIDE 8" MINIMUM BEARING ON BOTH SIDES.
3- EMBED VERTICAL REBARS INTO EXISTING WALL OR FOUNDATION TO REMAIN (DOWELS) W/ HILTI HIT RE 500 EPOXY 4" MIN.
4- COORDINATE LOCATION OF MASONRY WALL OPENINGS WITH ARCHITECTURAL DRAWINGS.

DEMOLITION KEY NOTES

1. EXISTING COLUMN TO REMAIN.
2. CONCRETE SLAB TO BE DEMOLISHED.
3. EXISTING PARTIAL WALL TO BE REMOVED. REFER TO ARCH. PLANS
4. EXTERIOR PAVEMENT TO BE DEMOLISHED.

DEMOLITION LEGEND

(diagonal hatch)	EXISTING TO REMAIN
(dashed dotted)	EXISTING STRUCTURE OR PARTIAL WALL AREA TO BE DEMOLISHED
(dashed crosshatch)	EXISTING CONCRETE SLAB AREA TO BE DEMOLISHED

OPPIDAN
builder of towns. creator of value.
Orchard SUPPLY HARDWARE

NAPLES NORTH
JOSE MIGUEL ORTIZ LICENSE No. 67920 STATE OF FLORIDA PROFESSIONAL ENGINEER
2/13/2017

cph www.cphcorp.com A Full Service A & E Firm
Eng. C.O.A. No. 3215 - 500 West Fulton Street Sanford, Florida 32771

ADDENDUM 02

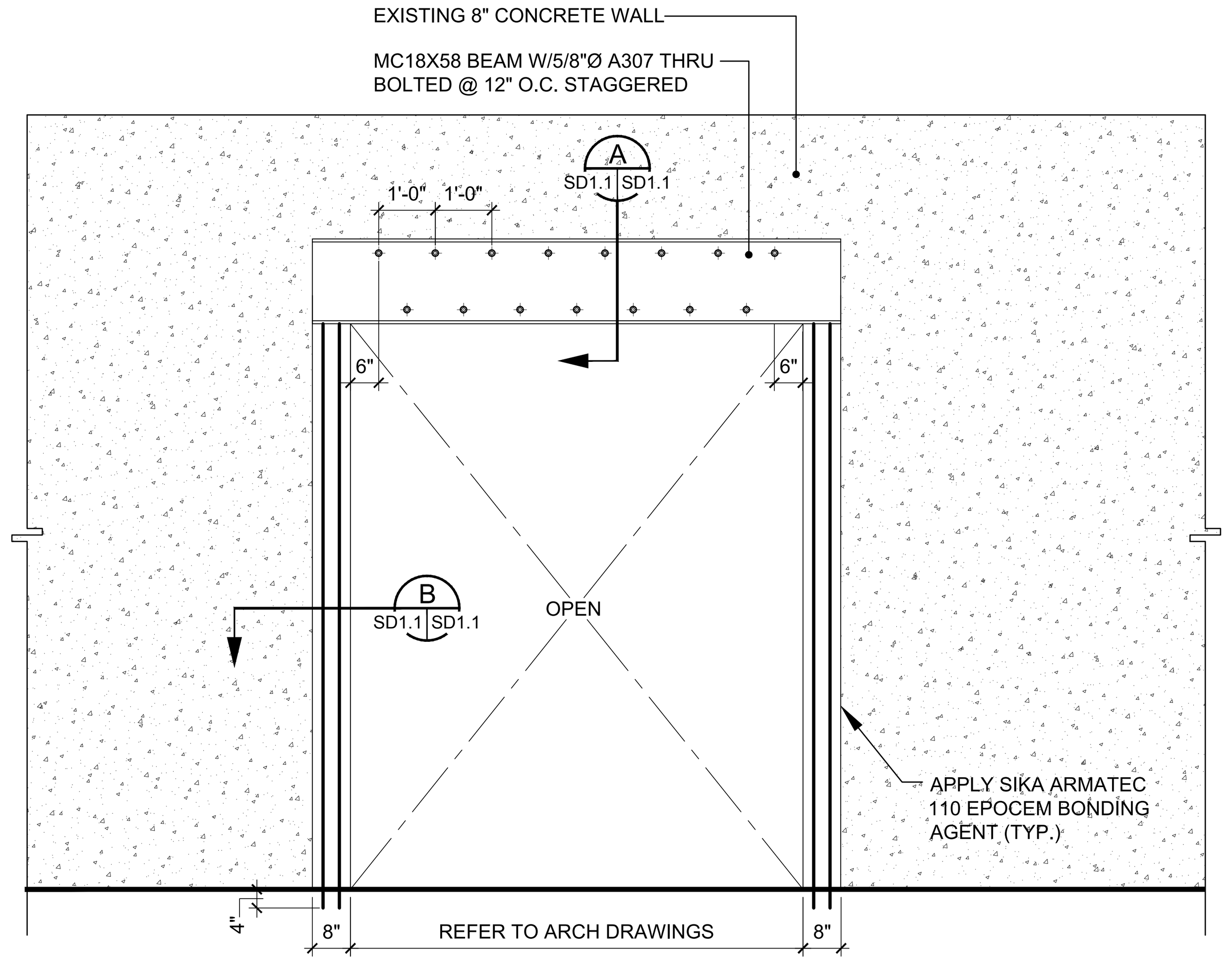
DEMOLITION PLAN

1000 IMMOKALEE ROAD, NAPLES, FL 34110

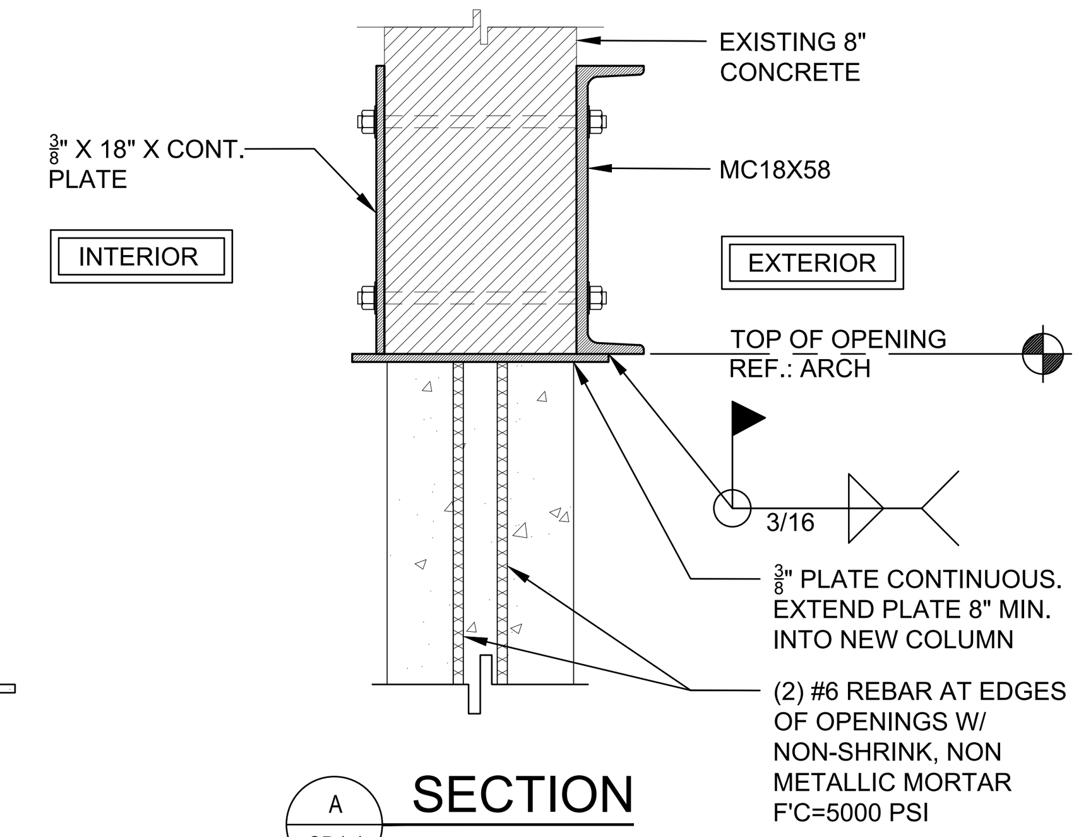
SD1.0
36-16117-00
11-21-2016
REVISIONS
2 2/08/2017 ADDENDUM 02

DLR Group
Architecture Engineering Planning Interiors

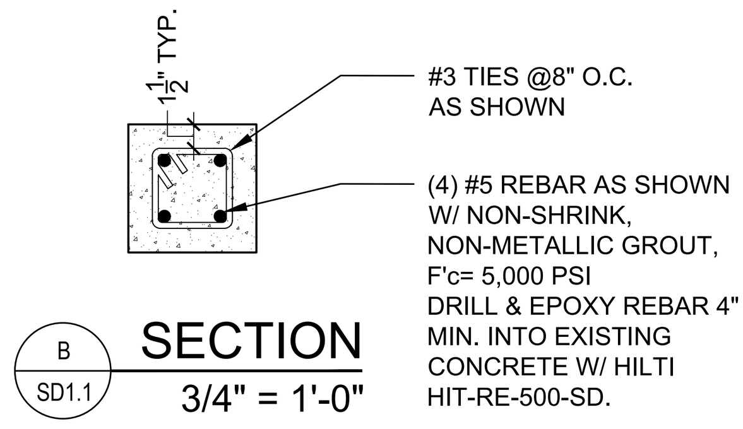
10/18/2016 8:43:16 AM



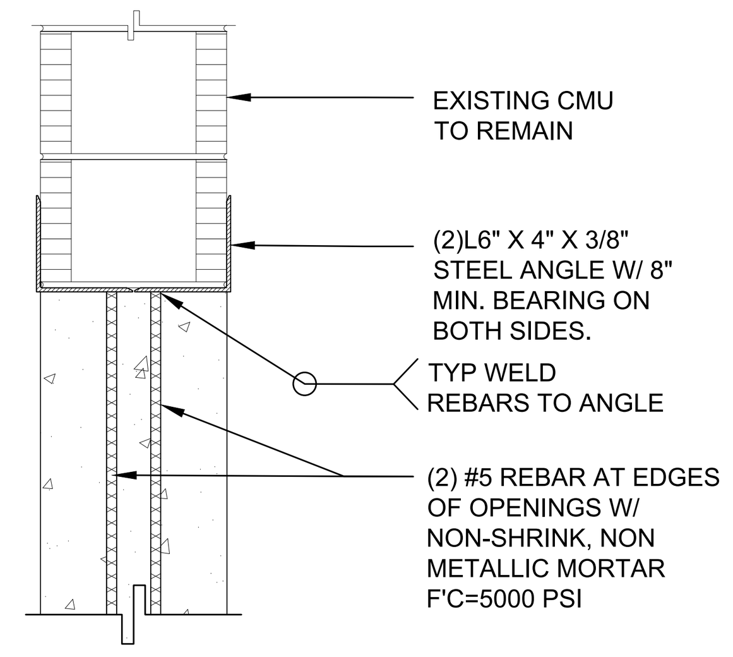
1 SD1.1 STRUCTURAL REINFORCEMENT
1/4" = 1'-0"



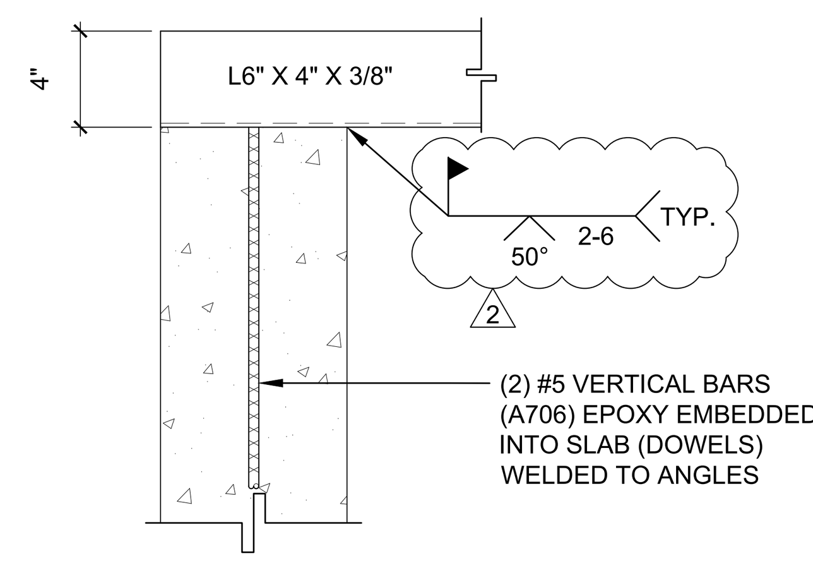
A SD1.1 SECTION
3/4" = 1'-0"



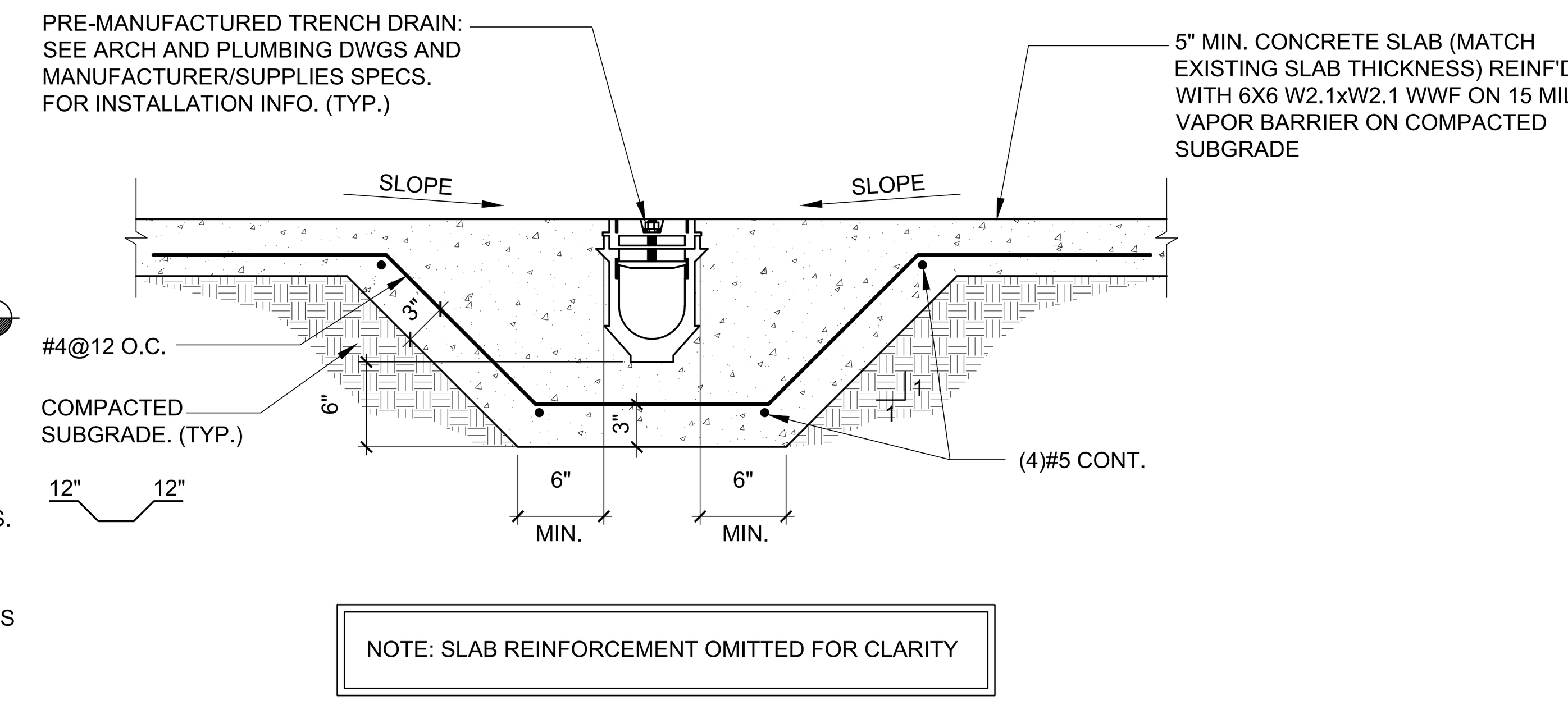
B SD1.1 SECTION
3/4" = 1'-0"



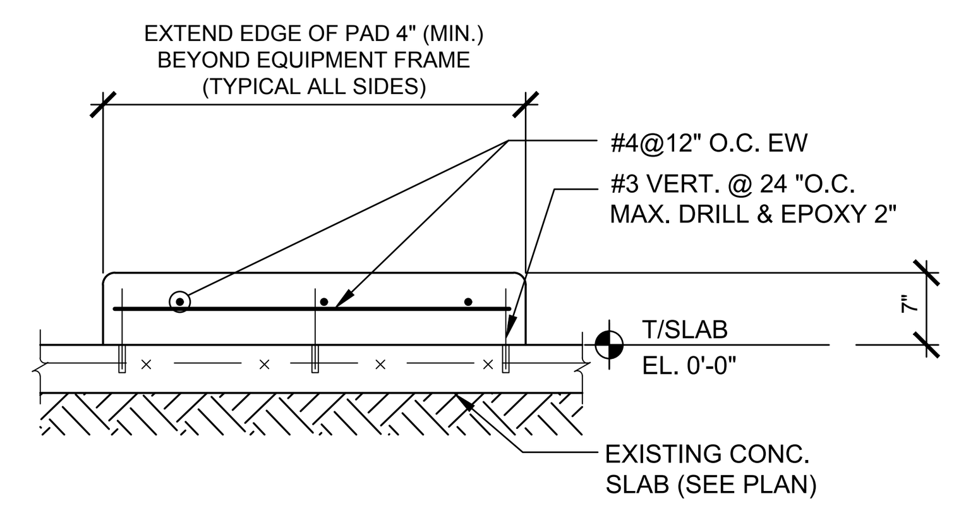
C SD1.1 TYPICAL HEAD DETAIL
1" = 1'-0"



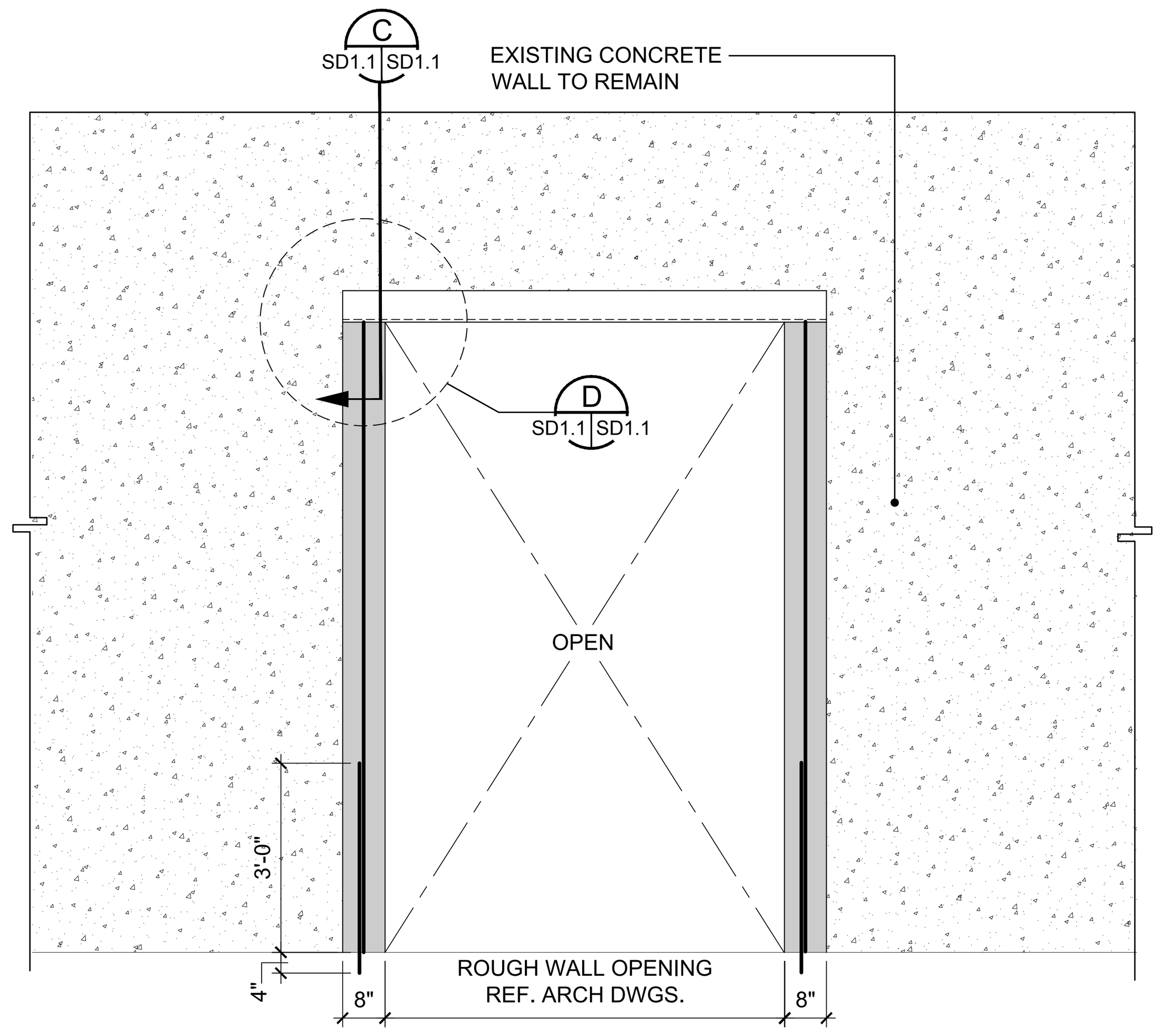
D SD1.1 SECTION
1" = 1'-0"



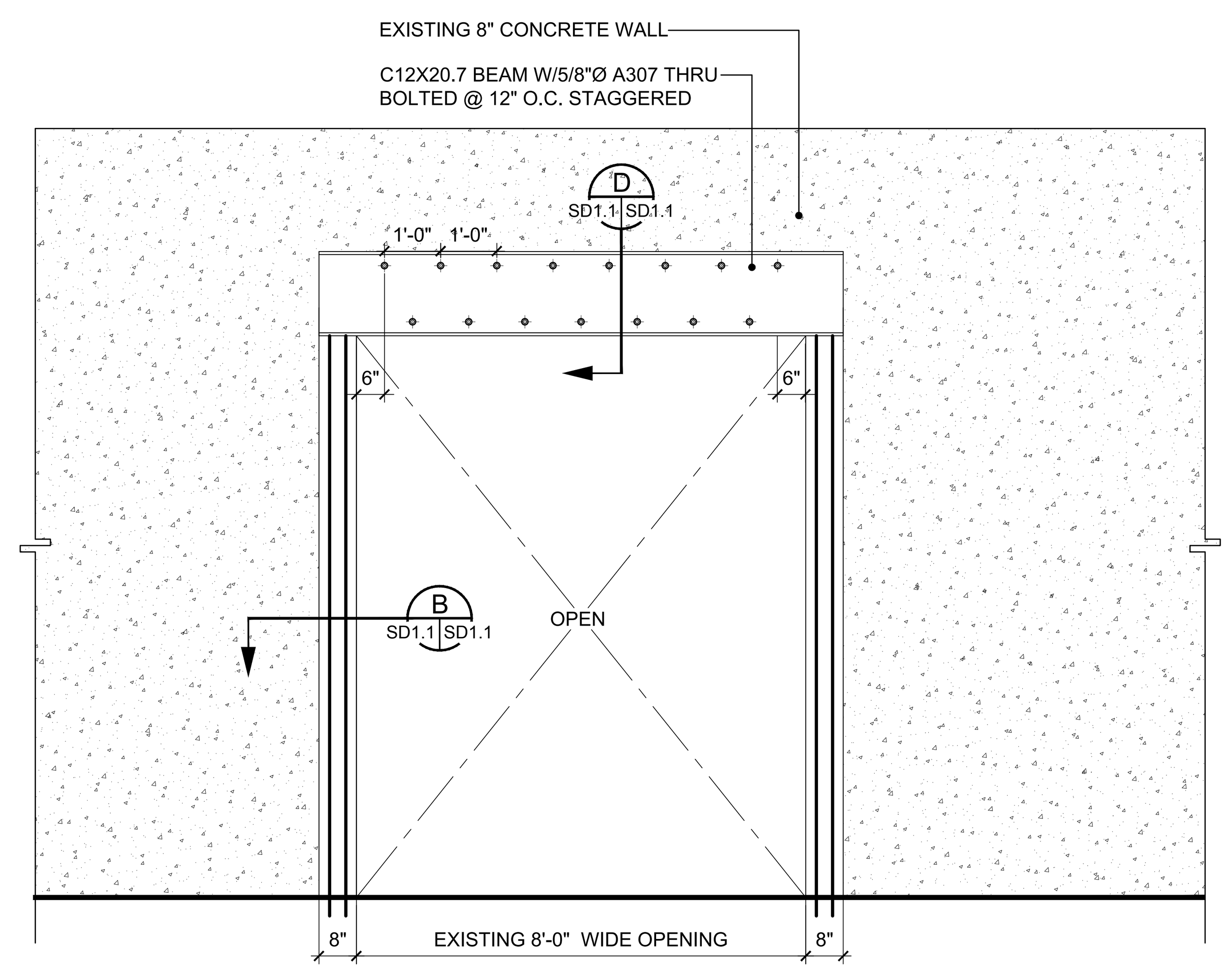
6 SD1.1 SD1.1 TYPICAL TRENCH DRAIN DETAIL
1" = 1'-0"



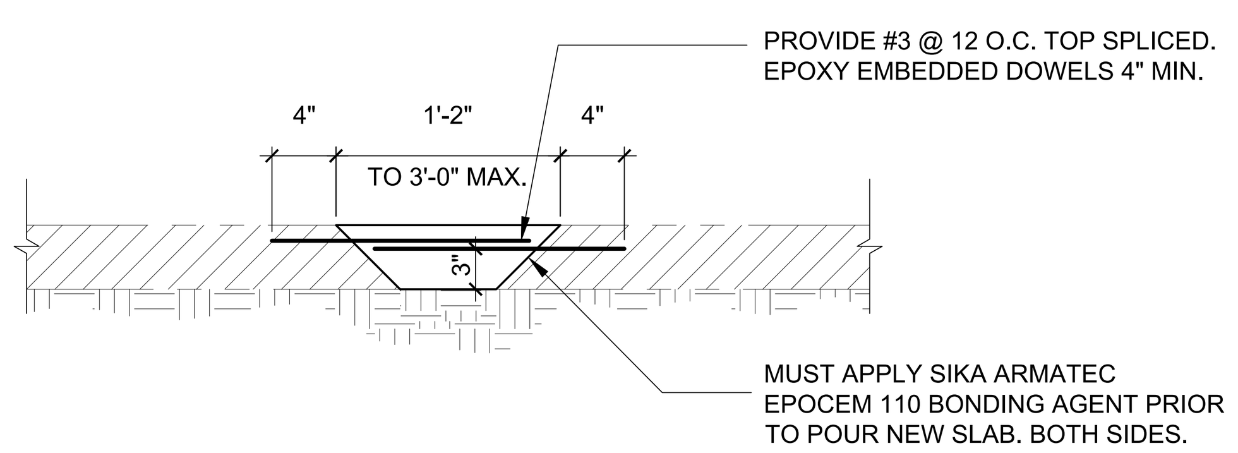
7 SD1.1 SD1.1 CONCRETE HOUSEKEEPING PAD DETAIL



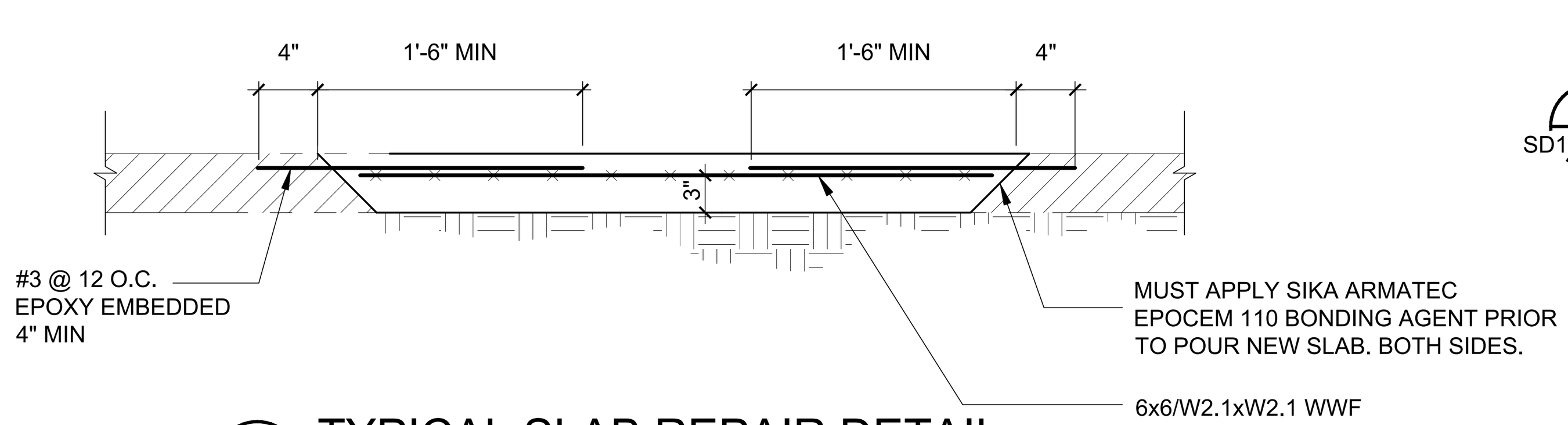
2 SD1.1 STRUCTURAL REINFORCEMENT
1/4" = 1'-0"



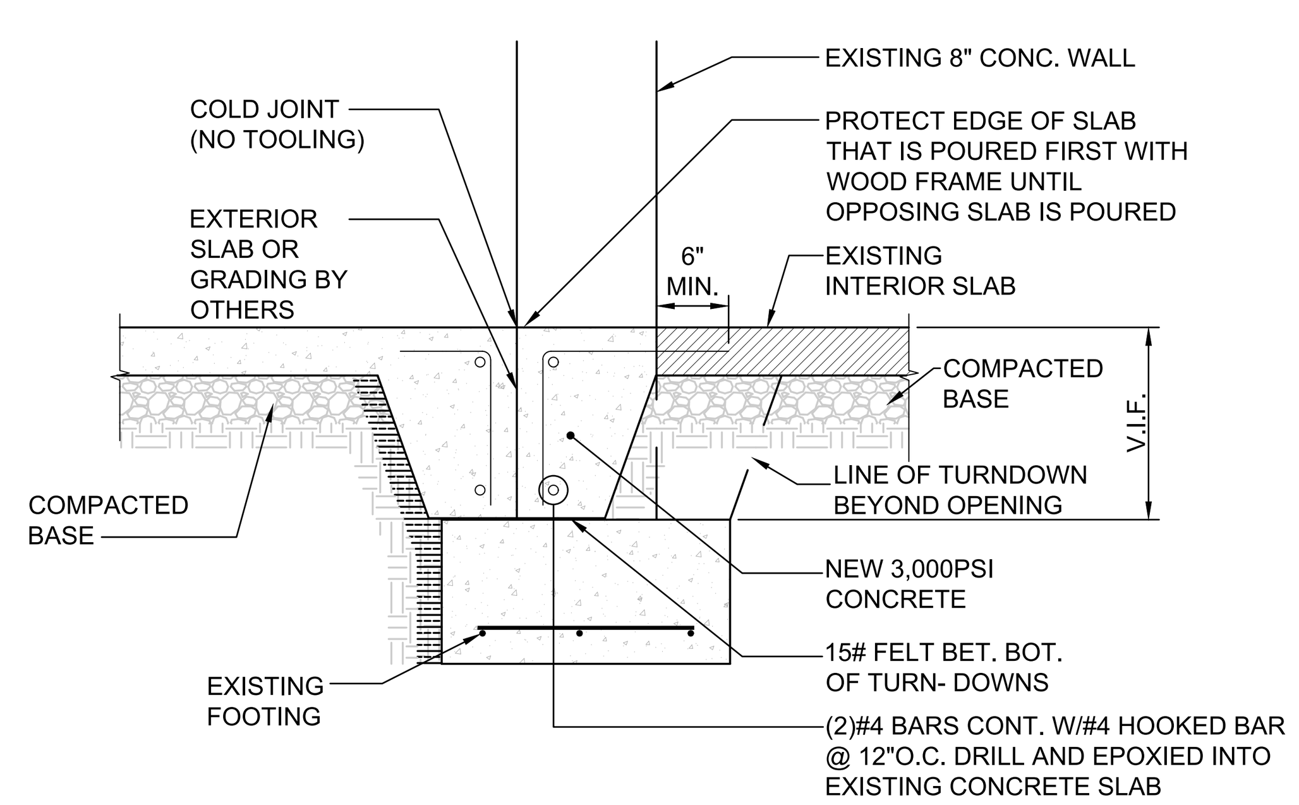
8 SD1.1 STRUCTURAL REINFORCEMENT
1/4" = 1'-0"



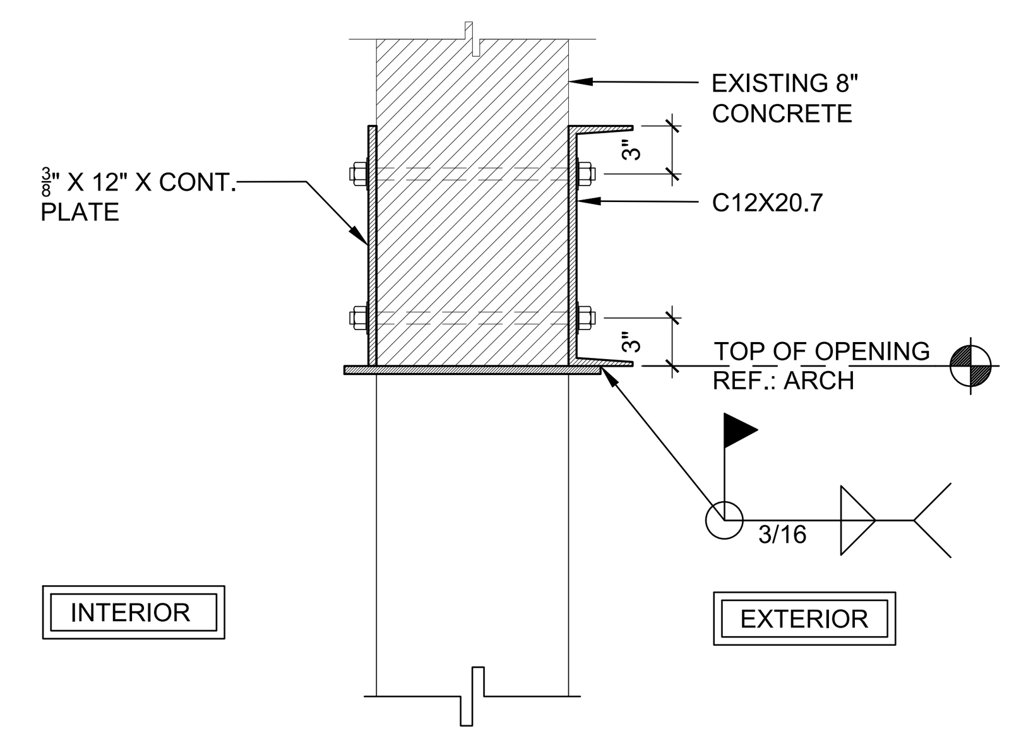
3 SD1.1 SD1.1 TYPICAL SLAB REPAIR DETAIL
1" = 1'-0"



4 SD1.1 SD1.1 TYPICAL SLAB REPAIR DETAIL
1" = 1'-0"



5 SD1.1 SD1.1 SECTION @ OPENINGS
3/4" = 1'-0"



D SD1.1 SECTION
3/4" = 1'-0"

OPPIDAN
builder of towns. creator of value.
Orchard SUPPLY HARDWARE

NAPLES NORTH

JOSE MIGUEL ORTIZ LICENSE No. 67920 STATE OF FLORIDA PROFESSIONAL ENGINEER
2/13/2017

cph
www.cphcorp.com
A Full Service A & E Firm
Eng. C.O.A. No. 3215 - 500 West Fulton Street Sanford, Florida 32771

ADDENDUM 02

DEMOLITION PLAN

1000 IMMOKALEE ROAD, NAPLES, FL 34110

SD1.1
36-16117-00
11-21-2016
REVISIONS
2 2/08/2017 ADDENDUM 02

DLR Group
Architecture Engineering Planning Interiors

10/18/2016 8:43:16 AM